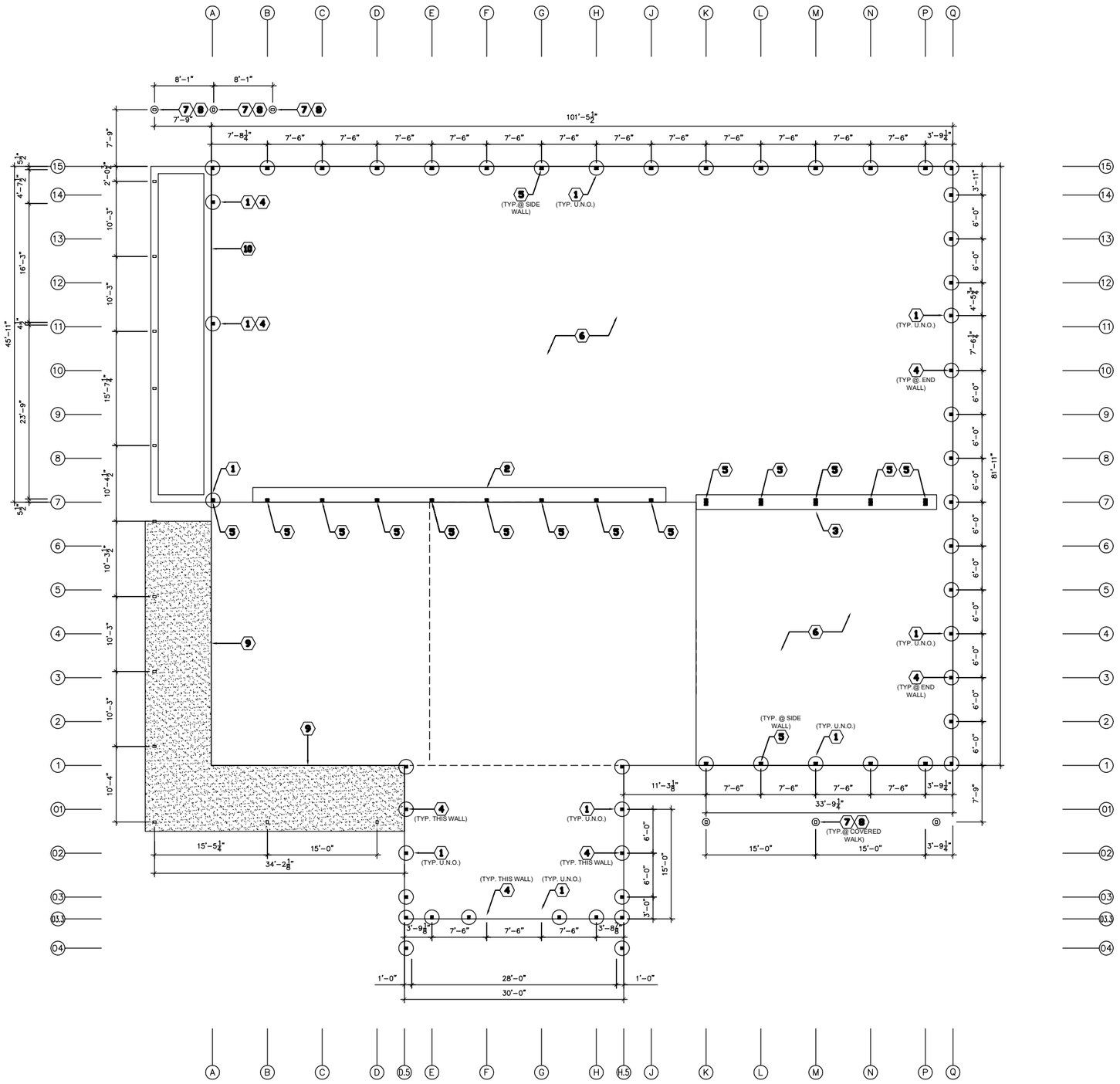

GENERAL NOTES:

FIELD VERIFY ALL DIMENSIONS AND ELEVATIONS, REPORT ALL DISCREPANCIES TO THE ARCHITECT.

c. REINFORCED CONCRETE

1. MATERIALS:
   - A. SPECIFICATIONS: IN GENERAL COMPLY WITH ACI 318-08 "BUILDING CODE AND COMMENTARY."
   - B. STRUCTURAL CONCRETE:

CLASS	LOCATION	F'c
I	FOOTINGS,CAISSONS, AND GRADE BEAMS	3000
II	INTERIOR SLABS ON GRADE AND ALL INTERIOR CONCRETE NOT OTHERWISE IDENTIFIED	3500
III	PIERS PLACED INTEGRALLY WITH WALLS, EXTERIOR SLABS ON GRADE, AND ALL EXTERIOR CONCRETE (WITH AIR) NOT OTHERWISE IDENTIFIED	4000
IV	BACKFILL BELOW FOOTINGS AND GRADE BEAMS	1500

   - C. ALL DEFORMED REINFORCING BARS: FY = 60,000 PSI
   - D. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A165-85. (SHEET FORM, NOT ROLLED)
2. FIELD MANUAL: PROVIDE AT LEAST ONE COPY OF THE ACI FIELD REFERENCE MANUAL, SP-15, IN THE FIELD OFFICE AT ALL TIMES.
3. CONTINGENCIES:
   - A. PROVIDE AND INSTALL _____ TON(S) OF REINFORCING BARS TO BE USED AS DIRECTED BY THE ENGINEER/ARCHITECT. COLD BEND IN THE FIELD, IF REQUIRED.
   - B. PROVIDE SUPPORTS AS REQUIRED TO MAINTAIN ALIGNMENT OF SCHEDULED REINFORCING. SUCH SUPPORTS ARE TO BE REFLECTED IN THE BID, AND ARE NOT PART OF THE CONTINGENCY QUANTITY LISTED ABOVE
4. OPENINGS:
   - A. OPENINGS SHOWN ARE FOR BIDDING PURPOSES ONLY. RECONCILE THEIR EXACT SIZES AND LOCATIONS WITH HVAC, PLUMBING, AND OTHER TRADES REQUIREMENTS BEFORE PROCEEDING WITH WORK.
   - B. IF ANY OPENING NOT SHOWN ON THE PLANSIS REQIRED, SECURE THE APPROVAL OF THE STRUCTURAL ENGINEER BEFORE PROCEEDING.
   - C. PROVIDE TWO (2) #5 BARS AROUND ALL SLAB AND WALL OPENINGS EXTENDING 2 INCHES BEYOND THE OPENING IN ALL DIRECTIONS. OPENINGS NOT EXCEEDING 16 INCHES X 16 INCHES MAY BE SLEEVED AS REQUIRED BY WORKING THE REINFORCING STEEL AROUND THEM
5. FOOTINGS, COLUMNS, WALLS:
   - A. DOWELS IN FOOTINGS TO MATCH VERTICAL COLUMN OR WALL REINFORCING.
   - B. UNLESS SHOWN OTHERWISE, PROVIDE THE FOLLOWING WALL REINFORCING:

WALL THICKNESS	HORIZONTAL STEEL	VERTICAL STEEL
6 & 8 INCH	#4 @ 12 INCHES	#4 @ 18 INCHES
10 & 12 INCH	#4 @ 16 INCHES E.F.	#4 @ 18 INCHES E.F.

   - C. PROVIDE CORNERBARS AT WALL CORNERS TO MATCH HORIZONTAL REINFORCING. MINIMUM LAP LENGTH WITH HORIZONTAL REINFORCEMENT IS 45 TIMES THE BAR DIAMETER.
   - D. DO NOT BACKFILL AGAINST BASEMENT WALLS UNTIL BOTH THE SLAB-ON-GRADE AND SUPPORT SLAB ABOVE ARE IN PLACE AND CURED.
   - E. BACKFILL AGAINST BOTH SIDES OF WALLS EQUALLY UNTIL THE LOWER ELEVATION IS ATTAINED.
   - F. PROVIDE MINIMUM 18 INCH THICK LAYER OF GRANULAR BACKFILL FULL HEIGHT OF ALL BASEMENT AND RETAINING WALLS.
   - G. CAST IN CONTINUOUS DOVETAIL ANCHOR SLOTS ON VERTICAL SURFACES WHERE MASONRY ABUTS, 16 INCHES O.C. FOR PARALELL SURFACES, AT CENTERLINE OF MASONRY FOR PERPENDICULAR SURFACES.
   - H. PROVIDE LEAN CONCRETE (CLASS IV) UNDER FOUNDATIONS FOR ACCIDENTAL OVEREXCAVATION, SOFT SPOTS, AND TRENCHES.
6. SPLICES: UNLESS NOTED OTHERWISE, MINIMUM LAP SPLICE LENGTHS TO BE AS FOLLOWS:

A. VERTICAL BARS IN WALLS, PIERS, OR COLUMNS (INCLUDING DOWELS)	30 DIAMETERS
B. HORIZONTAL BARS IN SLABS AND FOOTINGS	35 DIAMETERS
C. HORIZONTAL BARS IN WALLS	45 DIAMETERS

7. CONSTRUCTION JOINTS:
   - A. CONSTRUCTION JOINTS PERMITTED ONLY WHERE SHOWN OR APPROVED BY THE STRUCTURAL ENGINEER. ALL CONSTRUCTION JOINTS ARE TO BE KEYED. KEYWAYS SHALL BE 1-1/2 INCHES DEEP X 1/3 MEMBER THICNESS.
8. CONCRETE COVER: UNLESS NOTED OTHERWISE, DETAIL REINFORCING TO PROVIDE CONCRETE COVER AS FOLLOWS:

A. CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH	3 INCHES
B. CONCRETE EXPOSED TO EARTH OR WEATHER:	
#5 BARS AND SMALLER	1-1/2 IN.
OTHERS	2 INCHES
C. CONCRETE NOT EXPOSED TO EARTH OR WEATHER:	
BEAMS AND COLUMN BARS INCLUDING TIES, STIRRUPS, AND SPIRALS:	1-1/2 IN.
SLABS, WALLS, JOISTS #11 BARS AND SMALLER	2 INCHES
OTHERS	1-1/2 IN.

SHEET NOTES:

KEYED NOTES:

1. 12" THICK X 24" DIAMETER CONCRETE FOOTING, SEE DETAIL: S301-01
2. 18" THICK X 24" WIDE CONCRETE FOOTING W/ (3) #4 BARS CONTINUOUS HORIZONTAL, SEE DETAIL:S301-05.
3. 12" THICK X 24" WIDE CONCRETE FOOTING W/ (3) #4 BARS CONTINUOUS HORIZONTAL, SEE DETAIL:S301-04.
4. 3 PLY 2X6 LAMINATED COLUMN WITH #4 BAR ANCHOR ROD, SEE DETAIL S301-03.
5. 4 PLY 2X6 LAMINATED COLUMN WITH #4 BAR ANCHOR ROD, SEE DETAIL S301-03.
6. 4" THICK CONCRETE SLAB w/ 6x6 W2.9 x W2.9 WWF ON 6 MIL POLY VAPOR BARRIER ON 4" COMPACTED GRANNULAR FILL.
7. 6" THICK X 12" DIAMETER CONCRETE FOOTING, SEE DETAIL: S301-01 SIMILAR.
8. 4X6 PT. COLUMN WITH #4 BAR ANCHOR ROD, SEE DETAIL S301-03 SIMILAR.
9. EXISTING COVERED WALK TO REMAIN WHERE POSSIBLE.
10. 12" THICK X 32" DEEP TRENCH TURNDOWN SLAB FOUNDATION, SEE DETAILS305-04.

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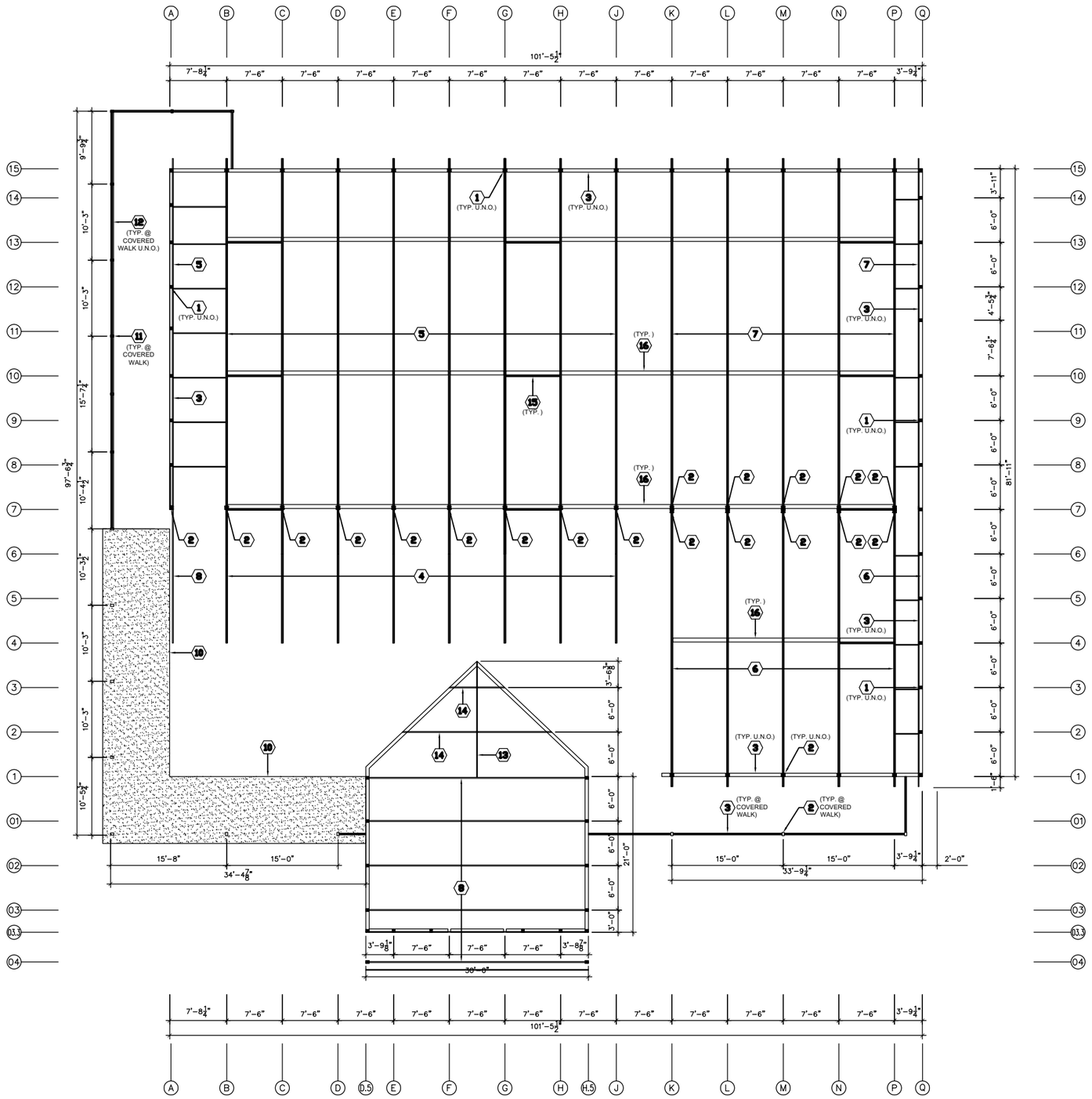
FOUNDATION PLAN

S101



GENERAL NOTES:

FIELD VERIFY ALL DIMENSIONS AND ELEVATIONS, REPORT ALL DISCREPANCIES TO THE ARCHITECT.

K. STRUCTURAL LUMBER:

1. A. SOUTHERN PINE #2:

SIZE	Fb	Ft	Fv	Fcl	Fc	E
2x4	1500	825	90	565	1650	1,600,000
2x6	1250	725	90	565	1600	1,600,000
2x8	1200	650	90	565	1550	1,600,000
2x10	1050	600	90	565	1500	1,600,000
2x12	975	550	90	565	1450	1,600,000

B. PLYWOOD: C-C PLUGGED, STRUCTURAL II, EXTERIOR GLUE. FOR ROOFS AND WALLS PANEL IDENTIFICATION INDEX 24/0 - 1/2 INCH OR 16/0 - 1/2 INCH (WITH PLYWOOD CLIPS)

2. SPECIFICATIONS: UNLESS SPECIFICALLY SHOWN OTHERWISE, DESIGN, FABRICATION, AND ERCTION SHALL BE GOVERNED BY THE LATEST REVISIONS OF:
   A. NATIONAL DESIGN SPECIFICATION FOR STRESS-GRADE LUMBER AND ITS FASTENINGS.
   B. U.S. PRODUCT STANDARD PS-1 FOR SOFTWOOD PLYWOOD - CONSTRUCTION AND INDUSTRIAL
3. CONNECTIONS:
   A. JOIST TO BEAMS - 16 GAGE GALVANIZED STANDARD JOIST HANGERS, UNLESS NOTED OTHERWISE.
   B. PLYWOOD TO ROOF TRUSSES OR RAFTERS - NAILED - USE 6d RING SHANK NAILS AT 6 INCHES O.C. AT PANEL EDGES AND 12 INCHES O.C. AT INTERMEDIATE SUPPORTS. PROVIDE PLYWOOD CLIPS AT MIDSPAN OF PLYWOOD BETWEEN SUPPORTS.
4. MISCELANEOUS:
   A. USE ONE LINE OF SOLID BLOCKING OR CROSS BRIDGING AT 8'-0" O.C. MAX FOR ALL JOISTS AND RAFTERS, USE SOLID BLOCKING AT JIOST AND RAFTER BEARING.
   B. USE SOLID BLOCKING AT MID-HEIGHT FOR ALL EXTERIOR STUD WALLS AND INTERIOR BEARING PARTITIONS.
   C. USE DOUBLE STUDS UNDER BEAM AND LINTEL BEARING, UNLESS NOTED OTHERWISE.

L. PREFABRICATED WOOD TRUSSES:

1. MATERIALS:
   A. LUMBER: SOUTHERN PINE #2: Fb = 1500 (REPETATIVE USE) PSI, Ft = 675 PSI, Fc (PARALELL TO GRAIN) = 1150 PSI, Fc (PERPENDICULAR TO GRAIN) = 565 PSI, E = 1600 KSI MAX., M.C. = 15%.
   B. METAL CONNECTOR PLATES: GALVANIZED SHEET STEEL ASTM A653/A653M-11 GRADE A, COATING CLASS G60 PER ASTM A653/A653M-11. MANUFACTURED WITH HOLES, PLUGS, TEETH, OR PRONGS UNIFORMLY SPACED AND FORMED.
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      4. NET WIND UPLIFT: 8 PSF
   B. DESIGN OF MEMBERS AND CONNECTIONS IS TO BE BY A PROFESSIONAL ENGINEER, REGISTERED IN OHIO. EXPERIENCED IN SIMILAR DESIGN, RETAINED BY THE MANUFACTURER.
   C. SHOP DRAWINGS SHALL EXHIBIT THE SEAL OF THE ENGINEER RESPONSIBLE FOR THE TRUSS DESIGN. IN ADDITION, DESIGN CALCULATIONS FOR THESE TRUSSES SHALL BE SUBMITTED TO THE ARCHITECT FOR REVIEW.
   D. MEMBER SIZES SHOWN ARE MINIMUM SIZES.
   E. MAXIMUM LIVE LOAD DEFLECTION IS TO BE L/360.
   F. MAXIMUM TOTAL LOAD DEFLECTION IS TO BE L/240.
3. MISCELANEOUS:
   A. BOLT TOP CHORDS OF ALL MULTIPLE MEMBER TRUSSES TOGETHER WITH 3/4" DIAMETER BOLTS AT 4'-0" O.C.. BOLT WEB MEMBERS TOGETHER WITH 34"
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   D. IN AREAS WHERE TOP CHORD OF TRUSSES DO NOT RECIEVE PLYWOOD SHEETING, PROVIDE 1x4 CONTINUOUS BRIDGING PERPENDICULAR TO TOP CHORDS AND SPACED AT 3'-0" O.C.
   E. TRUSS FABRICATOR SHALL SUBMIT COPIES OF THE FINAL APPROVED FABRICATION DRAWINGS TO THE OHIO DEPARTMENT OF INDUSTRIAL RELATIONS, DIVISION OF FACTORY AND BUILDING INSPECTION, PRIOR TO FABRICATION AND ERECTION.

SHEET NOTES:

KEYED NOTES:

1. 3 PLY 2X6 LAMINATED COLUMN WITH #4 BAR ANCHOR ROD, SEE DETAIL: S301-03.
2. 4 PLY 2X6 LAMINATED COLUMN WITH #4 BAR ANCHOR ROD, SEE DETAIL: S301-03.
3. 2x6 INSET GIRTS AT 24" O.C. VERTICAL, SEE DETAIL: S303-04.
4. 2 PLY 1-1/2 X 7-1/4 LVL RAFTERS @ 7'6" O.C. SEE DETAIL: S302-01.
5. PRE-ENGINEERED A TRUSSES @ 7'6" O.C. SEE DETAIL: S302-02
6. PRE-ENGINEERED B TRUSSES @ 7'6" O.C. SEE DETAIL: S302-03
7. PRE-ENGINEERED C TRUSSES @ 7'6" O.C. SEE DETAIL: S302-04
8. PRE-ENGINEERED D TRUSSES @ 6'0" O.C. SEE DETAIL: S302-05.
9. 2X4 ENDWALL FRAMING.
10. EXISTING COVERED WALK TO REMAIN WHERE POSSIBLE.
11. 4X6 PT COLUMN, SEE DETAIL: S305-06.
12. (3) PLY 1-1/2" X 5-1/2" 2X6 BEARING BEAM, SEE DETAIL: S305-06.
13. 1-3/4" X 11-1/4" LP-LVL 3100FB-2.2E RIDGE BEAM, OR APPROVED EQUIVALANT.
14. 2X6 RAFTERS @ 6' 0" O.C., SEE DETAIL: .
15. 2X6 CROSS BRACING, SEE DETAIL: S305-03.
16. 2X6 HORIZONTAL BRIDGING, SEE DETAIL: S305-03.

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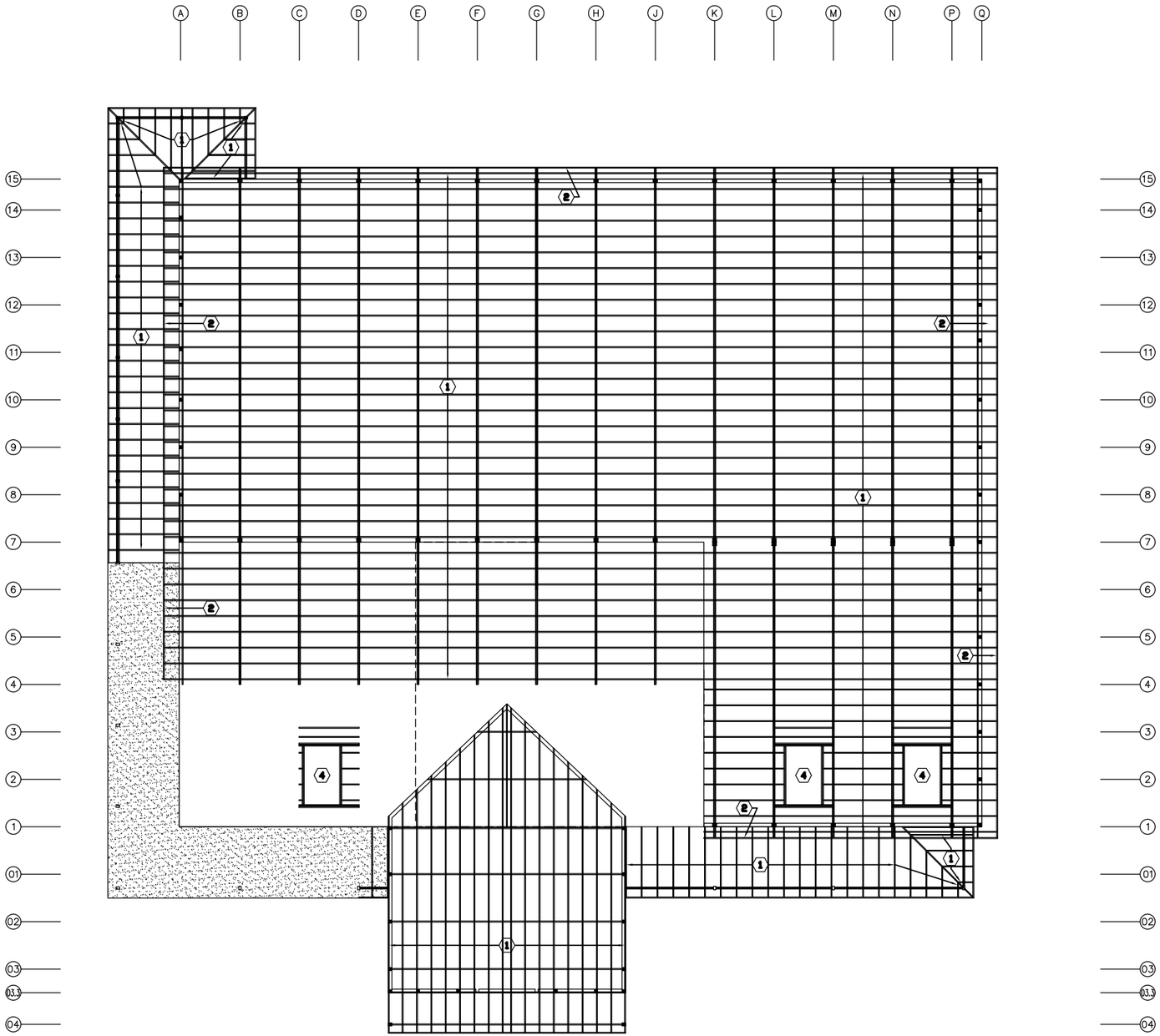
TRUSS FRAMING PLAN

S102



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SHEET NOTES:

KEYED NOTES:

1. 2x4 BYPASS PURLINS AT 24" O.C. VERTICAL, SEE DETAILS: S305-01 AND S305-02.
2. 2X6 FASCIA.
3. 2X6 RAFTERS @ 24" O.C..
4. DORMER SUPPORT FRAMING, SEE DETSIL: S303-08.

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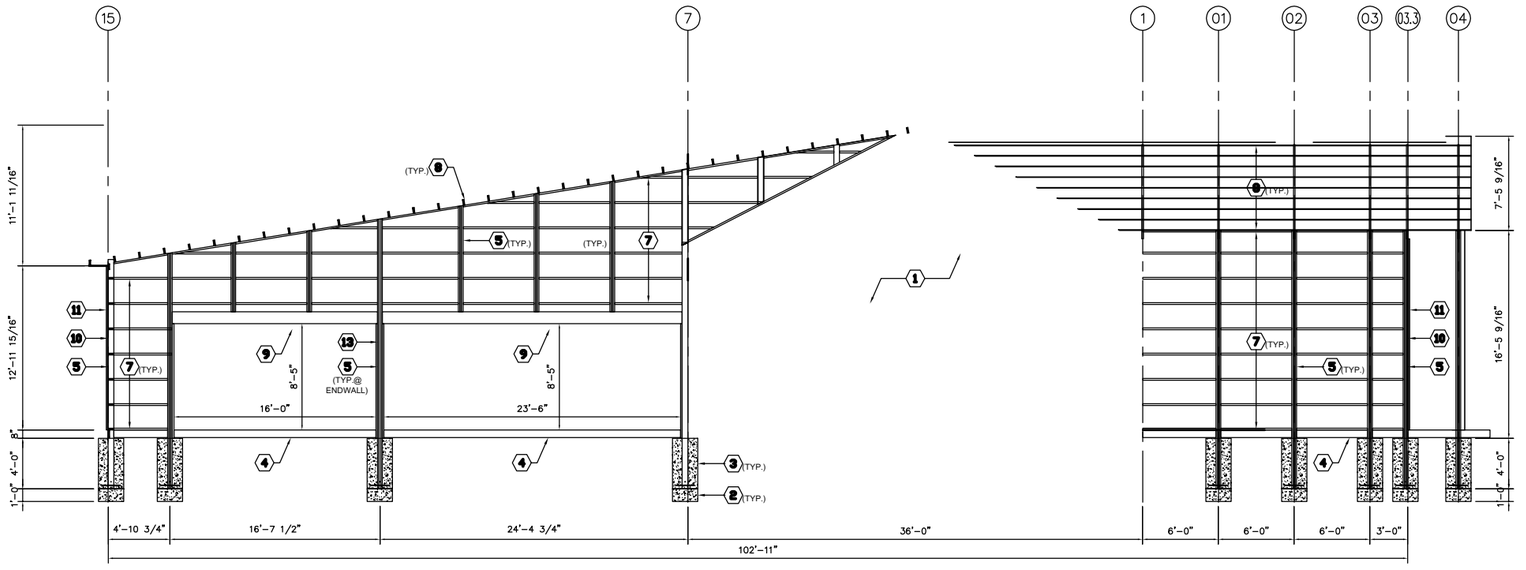
PURLIN FRAMING PLAN

S103



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SHEET NOTES:

KEYED NOTES:

1. EXISTING STRUCTURE
2. 12" THICK X 24" DIAMETER CONCRETE FOOTING WITH #4 REBAR MAT, SEE DETAIL: S303-01
3. 48" THICK X 24" DIAMETER CONCRETE FOUNDATION, SEE DETAIL: S303-01
4. 4" THICK CONCRETE SLAB w/ 6x6 W2.9 x W2.9 WWF ON 6 MIL POLY VAPOR BARRIER ON 4" COMPACTED GRANNULAR FILL.
5. 3 PLY 2X6 LAMINATED COLUMN WITH #4 BAR ANCHOR ROD, TYP. @ ENDWALLS AND ENTRY SIDEWALLS.
6. 4 PLY 2X6 LAMINATED COLUMN WITH #4 BAR ANCHOR ROD, TYP. @ 2 PLY TRUSS BEARING.
7. 2x6 INSET GIRTS AT 24" O.C. VERTICAL, SEE DETAIL: S303-04.
8. 2x4 BYPASS PURLINS AT 24" O.C. VERTICAL, SEE DETAIL S303-03.
9. (3) 2X12 BEAM
10. 1/5" OSB SHEETING.
11. METAL WALL PANEL.
12. MTEAL ROOF PANEL.
13. 2X6 JACK STUD, TYPICAL AT BEAM BEARING.

BROWN & BILLS
ARCHITECTS

55 HILLSIDE COURT
ENGLEWOOD, OH 45322
937 832 1150

COVERED WAGON

607 NORTH MAIN STREET
UNION, OHIO

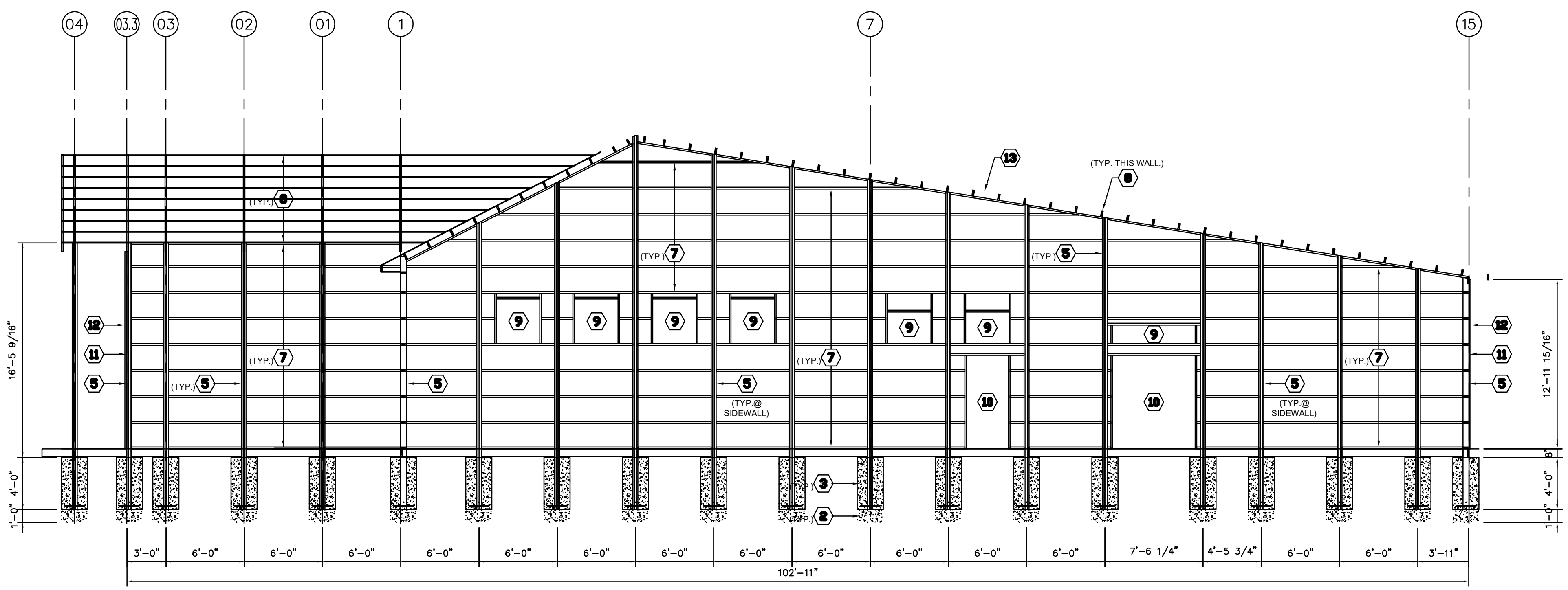
STATE OF OHIO
JONATHAN T. MILLER
E-68072
REGISTERED PROFESSIONAL ENGINEER

SUBMITTAL DATE
12/23/20

A201265

NORTH FRAMING ELEVATION

S201



GENERAL NOTES:

FIELD VERIFY ALL DIMENSIONS AND ELEVATIONS, REPORT ALL DISCREPANCIES TO THE ARCHITECT.

K. STRUCTURAL LUMBER:
1. A. SOUTHERN PINE #2:

SIZE	Fb	Ft	Fv	Fcl	Fc	E
2x4	1500	825	90	565	1650	1,600,000
2x6	1250	725	90	565	1600	1,600,000
2x8	1200	650	90	565	1550	1,600,000
2x10	1050	600	90	565	1500	1,600,000
2x12	975	550	90	565	1450	1,600,000

B. PLYWOOD: C-C PLUGGED, STRUCTURAL II, EXTERIOR GLUE. FOR ROOFS AND WALLS PANEL IDENTIFICATION INDEX 24/0 - 1/2 INCH OR 16/0 - 1/2 INCH (WITH PLYWOOD CLIPS)

2. SPECIFICATIONS: UNLESS SPECIFICALLY SHOWN OTHERWISE, DESIGN, FABRICATION, AND ERCTION SHALL BE GOVERNED BY THE LATEST REVISIONS OF:
   A. NATIONAL DESIGN SPECIFICATION FOR STRESS-GRADE LUMBER AND ITS FASTENINGS.
   B. U.S. PRODUCT STANDARD PS-1 FOR SOFTWOOD PLYWOOD - CONSTRUCTION AND INDUSTRIAL.
3. CONNECTIONS:
   A. JOIST TO BEAMS - 16 GAGE GALVANIZED STANDARD JOIST HANGERS, UNLESS NOTED OTHERWISE.
   B. PLYWOOD TO ROOF TRUSSES OR RAFTERS - NAILED - USE 6d RING SHANK NAILS AT 6 INCHES O.C. AT PANEL EDGES AND 12 INCHES O.C. AT INTERMEDIATE SUPPORTS. PROVIDE PLYWOOD CLIPS AT MIDSPAN OF PLYWOOD BETWEEN SUPPORTS.
4. MISCELANEOUS:
   A. USE ONE LINE OF SOLID BLOCKING OR CROSS BRIDGING AT 8'-0" O.C. MAX FOR ALL JOISTS AND RAFTERS, USE SOLID BLOCKING AT JIOST AND RAFTER BEARING.
   B. USE SOLID BLOCKING AT MID-HEIGHT FOR ALL EXTERIOR STUD WALLS AND INTERIOR BEARING PARTITIONS.
   C. USE DOUBLE STUDS UNDER BEAM AND LINTEL BEARING, UNLESS NOTED OTHERWISE.

L. PREFABRICATED WOOD TRUSSES:
1. MATERIALS:
   A. LUMBER: SOUTHERN PINE #2: Fb = 1500 (REPETATIVE USE) PSI, Ft = 675 PSI, Fc (PARALELL TO GRAIN) = 1150 PSI, Fc (PERPENDICULAR TO GRAIN) = 565 PSI, E = 1600 KSI MAX., M.C. = 15%.
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2. DESIGN CRITERIA:
   A. LOADING:
      1. TOP CHORD LIVE LOAD: 25 PSF
      2. TOP CHORD DEAD LOAD: 11 PSF
      3. BOTTOM CHORD DEAD LOAD: 7 PSF
      4. NET WIND UPLIFT: 8 PSF
   B. DESIGN OF MEMBERS AND CONNECTIONS IS TO BE BY A PROFESSIONAL ENGINEER, REGISTERED IN OHIO. EXPERIENCED IN SIMILAR DESIGN, RETAINED BY THE MANUFACTURER.
   C. SHOP DRAWINGS SHALL EXHIBIT THE SEAL OF THE ENGINEER RESPONSIBLE FOR THE TRUSS DESIGN. IN ADDITION, DESIGN CALCULATIONS FOR THESE TRUSSES SHALL BE SUBMITTED TO THE ARCHITECT FOR REVIEW.
   D. MEMBER SIZES SHOWN ARE MINIMUM SIZES.
   E. MAXIMUM LIVE LOAD DEFLECTION IS TO BE L/360.
   F. MAXIMUM TOTAL LOAD DEFLECTION IS TO BE L/240.
3. MISCELANEOUS:
A. BOLT TOP CHORDS OF ALL MULTIPLE MEMBER TRUSSES TOGETHER WITH 3/4" DIAMETER BOLTS AT 4'-0" O.C.. BOLT WEB MEMBERS TOGETHER WITH 34"
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   D. IN AREAS WHERE TOP CHORD OF TRUSSES DO NOT RECIEVE PLYWOOD SHEETING, PROVIDE 1x4 CONTINUOUS BRIDGING PERPENDICULAR TO TOP CHORDS AND SPACED AT 3'-0" O.C..
   E. TRUSS FABRICATOR SHALL SUBMIT COPIES OF THE FINAL APPROVED FABRICATION DRAWINGS TO THE OHIO DEPARTMENT OF INDUSTRIAL RELATIONS, DIVISION OF FACTORY AND BUILDING INSPECTION, PRIOR TO FABRICATION AND ERECTION.

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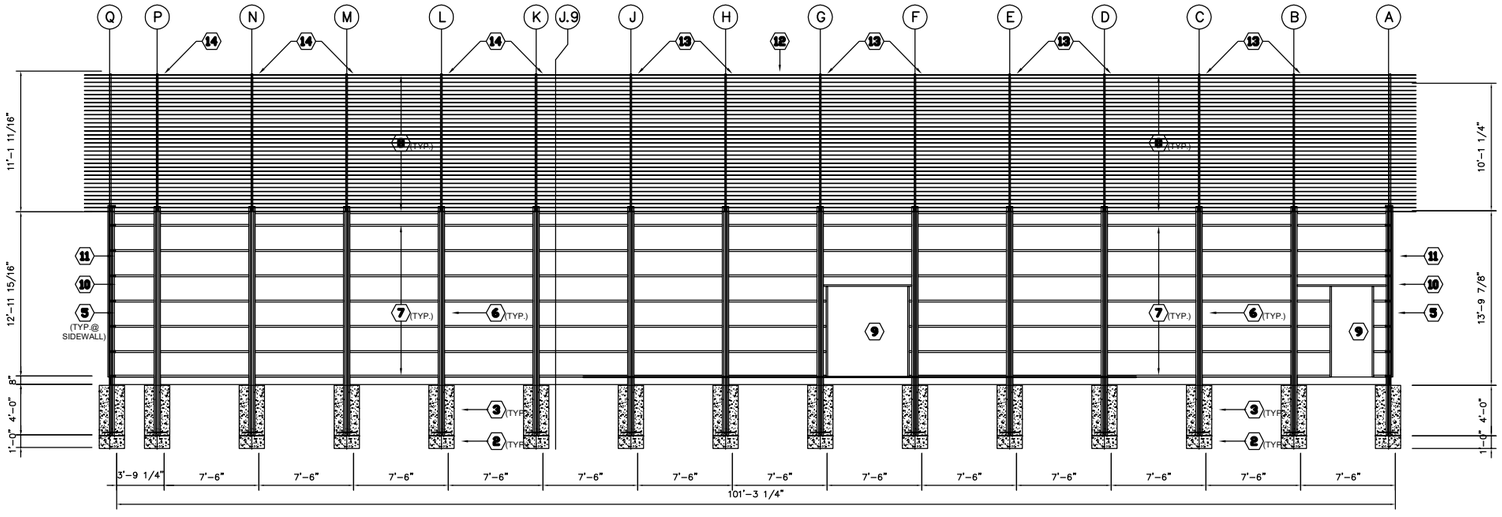
SOUTH FRAMING ELEVATION

S202



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10. 1/5" OSB SHEETING.
11. METAL WALL PANEL.
12. METAL ROOF PANEL.
13. (2) PLY PRE-ENGINEERED "A" TRUSSES @ 7'6" O.C. SEE DETAIL: S303-02.
14. (2) PLY PRE-ENGINEERED "C" TRUSSES @ 7'6" O.C. SEE DETAIL: S302-04.

BROWN & BILLS
ARCHITECTS

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ENGLEWOOD, OH 45322
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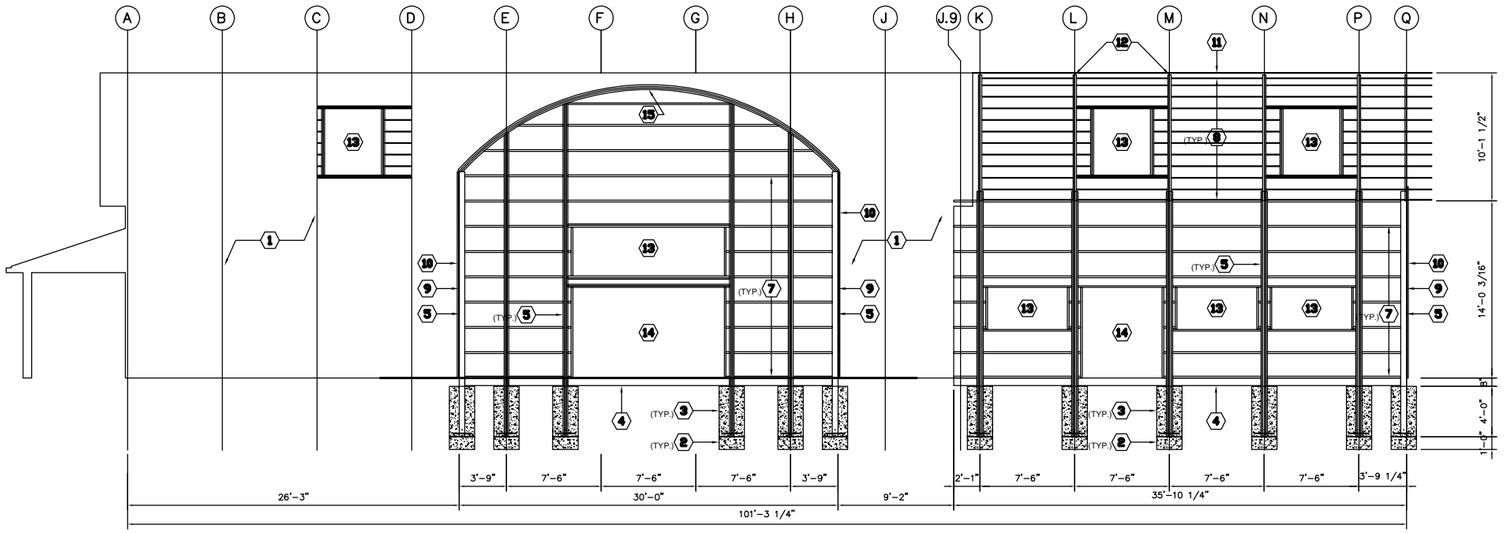
PROFESSIONAL ENGINEER

SUBMITTAL DATE
12/23/20

A201265

EAST FRAMING ELEVATION

S203



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5. 3 PLY 2X6 LAMINATED COLUMN WITH #4 BAR ANCHOR ROD, TYP. @ ENDWALLS AND ENTRY SIDEWALLS, SEE DETAIL S303-05.
6. 4 PLY 2X6 LAMINATED COLUMN WITH #4 BAR ANCHOR ROD, TYP. @ 2 PLY TRUSS BEARING, SEE DETAIL S303-05.
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12. (2) PLY PRE-ENGINEERED "B" TRUSSES @ 7'6" O.C. SEE DETAIL: S302-03.
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14. DOOR FRAMING, SEE DETAIL S303-08.
15. CURVED TRIPLE 2X6 TOP PLATE.

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ARCHITECTS

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12/23/20

A201265

WEST FRAMING ELEVATION

S204

A. GENERAL
1. THIS STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER THE BUILDING IS FULLY COMPLETED. IT IS SOLY THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ERECTION PROCEEDURE AND SEQUENCE AND TO ENSURE THE SAFETY OF THE BUILDING AND IT'S COMPONENT PARTS DURING ERECTION. THIS INCLUDES THE ADDITION OF WHATEVER SHORING, SHEETING, TEMPORARY BRACING, GUYS OR TIEDOWNS WHICH MIGHT BE NECESSARY. SUCH MATERIAL SHALL REMAIN THE CONTRACTOR'S PROPERTY AFTER THE COMPLETION OF THE PROJECT.
2. IT IS SOLELY THE CONTRACTOR'S RESPONSIBILITY TO FOLLOW ALL APPLICABLE SAFETY CODES AND REGULATIONS DURING ALL PHASES OF CONSTRUCTION.
3. EQUIPMENT FRAMING, LOADS, OPENINGS AND STRUCTURE IN ANY WAY RELATED TO HVAC, PLUMBING, OR ELECTRICAL REQUIREMENTS ARE SHOWN FOR BIDDING PURPOSES ONLY. CONTRACTOR SHALL OBTAIN APPROVAL OF THE INVOLVED TRADES BEFORE PROCEEDING WITH SUCH PORTION OF THE WORK. EXCESS COST RELATED TO VARIATION IN THESE REQUIREMENTS TO BE BORNE BY THE APPROPRIATE CONTRACTOR.
4. SHOULD ANY OF THE DETAILED INSTRUCTIONS SHOWN ON THE PLANS CONFLICT WITH THESE STRUCTURAL NOTES, THE SPECIFICATIONS, OR WITH EACH OTHER, THE STRICTIST PROVISION SHALL GOVERN.
5. ALL DIMENSIONS AND ELEVATIONS SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE VERIFIED BY THE CONTRACTOR AND SHALL CONFORM TO THOSE SHOWN ON THE ARCHITECTURAL DRAWINGS.
6. EXISTING BUILDING: PROVIDE TEMPORARY SUPPORTS AND OTHER MEASURES AS REQUIRED TO PREVENT DAMAGE TO THE EXISTING BUILDING DURING CONSTRUCTION. FIELD VERIFY ALL EXISTING DIMENSIONS AND ELEVATIONS THAT AFFECT THE NEW CONSTRUCTION.

B. GOVERNING CODE: 2011 OHIO BUILDING CODE.

1. ROOF SNOW LOADS:		
	A. GROUND SNOW LOAD	Pg = 20 PSF
	B. FLAT ROOF SNOW LOAD	Pf = 15.12 PSF
	C. SNOW EXPOSURE FACTOR	Ce = 0.9
	E. SNOW LOAD IMPORTANCE FACTOR	I = 1.0
	F. THERMAL FACTOR	Ct = 1.2
2. ROOF LIVE LOADS:		
	A. MINIMUM ROOF LIVE LOAD	Lr = 20 PSF
	B. COLLATERAL LOAD	Lc = 3 PSF
3. WIND LOADS:		
	A. BASIC WIND SPEED	V = 90 MPH
	B. OCCUPANCY CATEGORY	II
	C. WIND LOAD IMPORTANCE FACTOR	I = 1.0
	D. WIND EXPOSURE CATEGORY	C
	E. ENCLOSURE CATEGORY	ENCLOSED
	E. WIND DESIGN PRESSURE	q = 15.86 PSF
4. SEISMIC DESIGN DATA:		
	A. SPECTRAL RESPONSE SHORT PERIODS	Ss = 19.2%
	B. SPECTRAL RESPONSE 1 S PERIOD	S1 = 6.9%
	C. SEISMIC IMPORTANCE FACTOR	I = 1.0
	D. DESIGN CATEGORY	B
	E. SITE CLASS	D
	F. SEISMIC RESISTING SYSTEM:	
	1. LONGITUDINAL DIRECTION STEEL SYSTEM	R = 3.0
	2. LONGITUDINAL DIRECTION STEEL SYSTEM	R = 3.0
	G. SEISMIC RESPONSE COEFICIENT	Cs = 0.068
	H. SPECTRAL RESPONSE PARAMETER SHORT PERIOD	SDS = 0.205
	I. SPECTRAL RESPONSE PARAMETER 1 S PERIOD	SD1 = 0.110
	J. ANALYSIS PROCEDURE	E.L.F.

C. REINFORCED CONCRETE
1. MATERIALS:
A. SPECIFICATIONS: IN GENERAL COMPLY WITH ACI 318-08 "BUILDING CODE AND COMMENTARY."
B. STRUCTURAL CONCRETE:

CLASS	LOCATION	F'c
I	FOOTINGS,CAISSONS, AND GRADE BEAMS	3000
II	INTERIOR SLABS ON GRADE AND ALL INTERIOR CONCRETE NOT OTHERWISE IDENTIFIED	3500
III	PIERS PLACED INTEGRALLY WITH WALLS, EXTERIOR SLABS ON GRADE, AND ALL EXTERIOR CONCRETE (WITH AIR) NOT OTHERWISE IDENTIFIED	4000
IV	BACKFILL BELOW FOOTINGS AND GRADE BEAMS	1500

C. ALL DEFORMED REINFORCING BARS: FY = 60,000 PSI
D. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A165-85. (SHEET FORM, NOT ROLLED)
2. FIELD MANUAL: PROVIDE AT LEAST ONE COPY OF THE ACI FIELD REFERENCE MANUAL, SP-15, IN THE FIELD OFFICE AT ALL TIMES.
3. CONTINGENCIES:
A. PROVIDE AND INSTALL _____ TON(S) OF REINFORCING BARS TO BE USED AS DIRECTED BY THE ENGINEER/ARCHITECT. COLD BEND IN THE FIELD, IF REQUIRED.
B. PROVIDE SUPPORTS AS REQUIRED TO MAINTAIN ALIGNMENT OF SCHEDULED REINFORCING. SUCH SUPPORTS ARE TO BE REFLECTED IN THE BID, AND ARE NOT PART OF THE CONTINGENCY QUANTITY LISTED ABOVE
4. OPENINGS:
A. OPENINGS SHOWN ARE FOR BIDDING PURPOSES ONLY. RECONCILE THEIR EXACT SIZES AND LOCATIONS WITH HVAC, PLUMBING, AND OTHER TRADES REQUIREMENTS BEFORE PROCEEDING WITH WORK.
B. IF ANY OPENING NOT SHOWN ON THE PLANSIS REQUIRED, SECURE THE APPROVAL OF THE STRUCTURAL ENGINEER BEFORE PROCEEDING.
C. PROVIDE TWO (2) #5 BARS AROUND ALL SLAB AND WALL OPENINGS, EXTENDING 2 INCHES BEYOND THE OPENING IN ALL DIRECTIONS. OPENINGS NOT EXCEEDING 16 INCHES X 16 INCHES MAY BE SLEEVED AS REQUIRED BY WORKING THE REINFORCING STEEL AROUND THEM
5. FOOTINGS, COLUMNS, WALLS:
A. DOWELS IN FOOTINGS TO MATCH VERTICAL COLUMN OR WALL REINFORCING.
B. UNLESS SHOWN OTHERWISE, PROVIDE THE FOLLOWING WALL REINFORCING:

WALL THICKNESS	HORIZONTAL STEEL	VERTICAL STEEL
6 & 8 INCH	#4 @ 12 INCHES	#4 @ 18 INCHES
10 & 12 INCH	#4 @ 16 INCHES E.F.	#4 @ 18 INCHES E.F.

C. PROVIDE CORNERBARS AT WALL CORNERS TO MATCH HORIZONTAL REINFORCING. MINIMUM LAP LENGTH WITH HORIZONTAL REINFORCEMENT IS 45 TIMES THE BAR DIAMETER.
D. DO NOT BACKFILL AGAINST BASEMENT WALLS UNTIL BOTH THE SLAB-ON-GRADE AND SUPPORT SLAB ABOVE ARE IN PLACE AND CURED.
E. BACKFILL AGAINST BOTH SIDES OF WALLS EQUALLY UNTIL THE LOWER ELEVATION IS ATTAINED.
F. PROVIDE MINIMUM 18 INCH THICK LAYER OF GRANULAR BACKFILL FULL HEIGHT OF ALL BASEMENT AND RETAINING WALLS.
G. CAST IN CONTINUOUS DOVETAIL ANCHOR SLOTS ON VERTICAL SURFACES WHERE MASONRY ABUTS, 16 INCHES O.C. FOR PARALELL SURFACES, AT CENTERLINE OF MASONRY FOR PERPENDICULAR SURFACES.
H. PROVIDE LEAN CONCRETE (CLASS IV) UNDER FOUNDATIONS FOR ACCIDENTAL OVEREXCAVATION, SOFT SPOTS, AND TRENCHES.
6. SPLICES: UNLESS NOTED OTHERWISE, MINIMUM LAP SPLICE LENGTHS TO BE AS FOLLOWS:
A. VERTICAL BARS IN WALLS, PIERS, OR COLUMNS
30 DIAMETERS
(INCLUDING DOWELS)
B. HORIZONTAL BARS IN SLABS AND FOOTINGS
35 DIAMETERS
C. HORIZONTAL BARS IN WALLS
45 DIAMETERS
7. CONSTRUCTION JOINTS:
A. CONSTRUCTION JOINTS PERMITTED ONLY WHERE SHOWN OR APPROVED BY THE STRUCTURAL ENGINEER. ALL CONSTRUCTION JOINTS ARE TO BE KEYED. KEYWAYS SHALL BE 1-1/2 INCHES DEEP X 1/3 MEMBER THICKNESS.
8. CONCRETE COVER: UNLESS NOTED OTHERWISE, DETAIL REINFORCING TO PROVIDE CONCRETE COVER AS FOLLOWS:
A. CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH 3 INCHES
B. CONCRETE EXPOSED TO EARTH OR WEATHER:
#5 BARS AND SMALLER
1-1/2 IN.
OTHERS
2 INCHES
C. CONCRETE NOT EXPOSED TO EARTH OR WEATHER:
BEAMS AND COLUMN BARS INCLUDING TIES, STIRRUPS, AND SPIRALS
1-1/2 IN.
SLABS, WALLS, JOISTS #11 BARS AND SMALLER
2 INCHES
OTHERS
1-1/2 IN.

D. PRE-ENGINEERED STEEL BUILDING:
1. DESIGN, FABRICATION, AND ERECTION PER 2006 MBMA METAL BUILDING SYSTEMS MANUAL, WITH 2010 SUPPLEMENT TO 2006 METAL BUILDING SYSTEMS MANUAL.
2. DESIGN OF MEMBERS AND CONNECTIONS IS TO BE BY A PROFESSIONAL ENGINEER, REGISTERED IN OHIO, EXPERIENCED IN SIMILAR DESIGN, RETAINED BY THE MANUFACTURER. THE DESIGN ENGINEER OF THE METAL BUILDING SYSTEM SHALL BE THE ENGINEER OF RECORD OF THE METAL BUILDING SYSTEM AND RETAIN LIABILITY FOR THE METAL BUILDING SYSTEM.
3. SHOP DRAWINGS SHALL EXHIBIT THE SEAL OF THE ENGINEER RESPONSIBLE FOR THE DESIGN. IN ADDITION, DESIGN CALCULATIONS FOR THE METAL BUILDING SYSTEM SHALL BE SUBMITTED TO THE ARCHITECT FOR REVIEW.

E. STRUCTURAL STEEL:
1. MATERIALS:
A. STRUCTURAL STEEL: ASTM A36, Fy = 36 KSI; ASTM A572, Fy = 50 KSI; HIGH STRENGTH BOLTS: ASTM A325 OR A490; ANCHOR BOLTS: ASTM A307 OR A36; ELECTRODES: SERIES E70; SRTUCTURAL PIPES: ASTM A53 OR A501, Fy = 35 KSI MIN.; SQUARE AND RECTANGULAR TUBING: ASTM A500, Fy = 46 KSI; EXPANSION BOLTS: HILTI "KWIK BOLTS" OR APROVED EQUAL.
2. SPECIFICATIONS: WELDING PERSONNEL AND PROCEEDURES ARE TO BE QUALIFIED PER AWS D1.1. UNLESS SPECIFICALLY SHOWN OTHERWISE, DESIGN, FABRICATION, AND ERECTION TO BE GOVERNED BY:
A. AISC SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS (MARCH 9, 2005)
B. AICS CODE OF STANDARD PRACTICE (MARCH 18, 2005)
C. STRUCTURAL WELDING CODE, AWS D1.1/D1.1M:2006 OF THE AMERICAN WELDING SOCIETY.
D. SPECIFICATIONS FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS(DECEMBER 31, 2009)
3. CONNECTIONS:
A. FIELD CONNECTIONS TO BE BOLTED SHOP CONNECTIONS TO BE WELDED OR BOLTED. CONNECTIONS TO BE DESIGNED BY THE FABRICATOR TO DEVELOPE FULL STRENGTH OF MEMBER OR FORCES SHOWN ON THE PLANS, WHICH EVER GOVERNS. FOLLOW INSTRUCTIONS ON DRAWINGS FOR GENERAL ARRANGEMENT OR PARTICULAR DETAILS.
B. FULL PENETRATION AND PARTIAL PENETRATION FIELD WELDS IN MATERIAL OVER 5/16 INCH THICK SHALL BE SUBJECT TO NON-DESTRUCTIVE TESTING (OTHER THAN VISUAL INSPECTION) BY AN INDEPENDANT LABORATORY.
C. ALL BOLTS IN BRACED FRAMES AND BOLTS IN SHEAR CONNECTIONS USEN IN CONJUNCTION WITH FULL PENETRATION FLANGE WELDS SHALL BE SLIP CRITICAL (FRICTION) TYPE.
4. PAINT:
A. FINISHED PRODUCT CONCEALED FROM VIEW. DO NOT PAINT ANY STEEL WHICH WILL BE LOCATED OUT OF VIEW.
5. GALVANIZING: ALL SHELF ANGLES, LINTELS IN EXTERIOR WALLS, AND ALL EXTERIOR STEEL EXPOSED TO THE ELEMENTS SHALL BE GALVANIZED.
6. MISCELLANEOUS:
A. PROVIDE HOLES FOR OTHERS. IF OPENING IS NOT SHOWN ON THE STRUCTURAL DRAWINGS, OBTAIN PRIOR APPROVAL.
B. STEEL SUPPORTING OR CONNECTED TO HVAC AND OTHER EQUIPMENT AND ROOF OPENINGS AS SHOWN ON THE DRAWINGS IS SHOWN FOR BIDDING PURPOSES ONLY. CONTRACTOR SHALL RECONCILE EXACT SIZE AND LOCATION BEFORE PROCEEDING WITH HIS WORK.
C. GROUT UNDER BEARING PLATES, BASE PLATES, AND SETTING PLATES TO BE NON-SHRINKING TYPE.
D. STEEL BELOW GRADE TO BE PROTECTED BY A MINIMUM OF 3 INCHES OF CONCRETE.
E. PROVIDE 1/4" THICK SETTING PLATES FOR ALL BEAMS BEARING ON MASONRY WHICH DO NOT REQUIRE A BEARING PLATE.
F. PROVIDE SHOP WELDED ANCHORS FOR ATTACHMENTS OF MASONRY. SPACING TO BE 16 INCHES ON COLUMNS AND BEAMS.
G. PROVIDE HEAVY WASHER AT ALL ANCHOR BOLTS.
H. FINISH ENDS OF ALL COLUMNS, STIFFNERS, AND ALL OTHER MEMBERS IN DIRECT BEARING.
I. PROVIDE BOLT HOLES FOR JOISTS BOLTED TO BEAMS AND ATTACHMENT FOR JOINING EXTENDED JOIST BOTTOM CHORDS.
J. MINIMUM BEARING ON MASONRY = 8 INCHES UNLESS NOTED OTHERWISE.
K. EMBEDMENT LENGTH OF EXPANSION BOLTS INTO SOLID MASONRY OR CONCRETE SHALL BE AS FOLLOWS:
1/2 INCH DIAMETER BOLTS --- 3 1/2 INCHES EMBEDMENT
3/4 INCH DIAMETER BOLTS --- 5 INCHES EMBEDMENT

F. STEEL JOISTS
1. SPECIFICATIONS:
A. FABRICATION AND ERECTION PER SJI REQUIREMENTS.
B. MANUFACTURER TO BE A MEMBER OF SJI.
2. BRIDGING:
A. NUMBER OF ROWS AS SHOWN ON THE CONTRACT DRAWINGD, BUT NOT LESS THAT REQUIRED BY SJI. UNLESS NOTED OTHERWISE, USE HORIZONTAL BRIDGING FOR K-SERIES (EXCEPT USE A DIAGONAL ROW NEAREST THE MIDSPAN WHERE FOUR OR FIVE ROWS ARE SHOWN OR REQUIRED BY SJI).
B. HORIZONTAL BRIDGING MAY BE WELDED TO THE JOISTS.
C. DIAGONAL BRIDGING TO BE BOLTED TO THE JOISTS AND AT THEIR POINT OF INTERSECTION. ENDS OF DIAGONAL BRIDGING TO BE ANCHORED WITH HORIZONTAL BRIDGING UNLESS SHOWN OTHERWISE. HORIZONTAL BRIDGING IN NO MORE THAN TWO CONSECUTIVE BAYS MAY BE USED TO PRIVIDE PASSAGE FOR DUCT WORK.
D. ANCHOR BRIDGING TO INTERSECTING STRUCTURAL STEEL OR MASONRY WALLS.
3. BEARING:
A. WELD ALL JOISTS TO SUPPORTING STEEL WITH 1-1/2 INCHES OF 1/8 INCH FILLET WELD FOR K-SERIES JOISTS EACH SIDE OF BEARING. JOISTS TO BE FIELD BOLTED AT COLUMN LINES, OR IF THERE IS NO JOIST AT A COLUMN LINE, FIELD BOLT THE JOIST NEAREST THE COLUMN ON EACH SIDE. EXTEND BOTTOM CHORDS OF THE SAME JOISTS AND WELD THEM TO THE BEAM OR COLUMN.
B. EXTEND ALL JOISTS 1 INCH MINIMUM PAST CENTERLINE OF SUPPORTING MEMBER WHERE POSSIBLE. BEARINGS TO BE PER DRAWINGS, OR, WHERE SPECIAL INSTRUCTION IS NOT GIVEN, ACCORDING TO THE STANDARD SPECIFICATIONS OF SJI.
4. MISCELLANEOUS:
A. ADJACENT JOISTS OF THE SAME DEPTH ARE TO HAVE WEB MEMBERS LINE TO PERMIT PASSAGE OF HVAC DUCTS.
B. SEE DRAWINGS FOR SPECIAL BEARING SHOES, EXTENDED ENDS, LOAD DIAGRAMS, ETC..

G. STEEL ROOF DECK
1. REFERENCE: STEEL DECK INSTITUTE: "DESIGN MANUAL FOR COMPOSITE DECKS, FORM DECKS, AND ROOF DECKS - NO. 31, ANSI/SDI C-2011 STANDARD COMPOSITE STEEL FLOOR DECK - SLABS, ANSI/SDI RD-2010 STANDARD FOR STEEL ROOF DECK".
2. END JOINTS OF DECKS SHALL BE STAGGERED WITH 2 INCH MINIMUM ENR LAPS.
3. DECK SHALL BE 1-1/2 INCH 20 GAGE, WIDE RIB DECK, TYPE WR.
4. UNLESS NOTED OTHERWISE, ROOF DECK IS TO BE WELDED WITH 5/8 INCH DIAMETER PUDDLE WELDS AT 12 INCHES O.C. MAX. AT INTERIOR SUPPORTS AND 6 INCHES O.C. AT EDGE SUPPORT. INSTALL (1) #12 HEX HEAD SCREW PER DECK SPAN AT SHEET SIDELAPS.

H. WELDING:
1. REFERENCES:
A. AWS D1.1/D1.1M:2006 STRUCTURAL WELDING CODE - STEEL.
B. AWS D1.3/D1.3M:2008 STRUCTURAL WELDING CODE - SHEET STEEL.
2. ALL WELDING BY AWS QUALIFIED OPERATORS.

I. LIGHTGAGE METAL FRAMING:
1. MATERIALS:
A. STUDS AND TRACKS: 12, 14, AND 16 GAGE STUDS; ASTM A446 GRADE D, Fy = 50 KSI.

J. LIGHTGAGE METAL FRAMING:
1. MATERIALS:
A. STUDS AND TRACKS: 12, 14, AND 16 GAGE STUDS; ASTM A446 GRADE D, Fy = 50 KSI.
2. SPECIFICATIONS: WELDING PERSONNEL AND PROCEEDURES ARE TO BE QUALIFIED PER AWS. DESIGN, FABRICATION, AND ERECTION TO BE GOVERNED BY LATEST REVISIONS OF:
A. AISC SPECIFICATION OF THE DESIGN OF COLD FORM STEEL STRUCTURAL MEMBERS.
B. AWS D1.3/D1.3M:2008 STRUCTURAL WELDING CODE - SHEET STEEL.
3. PROPERTIES:
A. LIGHTGAGE FRAMING MEMBER SIZES INDICATED ON DRAWINGS SHALL MEET THE FOLLOWING MINIMUM REQUIREMENTS:

MEMBER DESIGNATION	DEPTH	FLANGE WIDTH	GAGE	Ix (IN4)	Mr (IN-LB)
C6 X 16	6"	1 5/8"	16	2.958	21,152
C8 X 16	8"	1 5/8"	16	5.960	38,412
C10 X 14	10"	1 5/8"	14	12.800	66,316
C12 X 14	12"	1 5/8"	14	20.295	87,030
C12 X 16	12"	1 5/8"	16	16.417	73.583

4. CONNECTIONS:
A. ALL CONNECTIONS TO BE FIELD WELDED.
5. FINISH:
A. ALL MATERIALS TO BE GALVANIZED COATED IN ACCORDANCE WITH ASTM A525 G-60
B. TOUCH UP FIELD WELDS WITH ZINC RICH PAINT.
6. MISCELLANEOUS:
A. SUBMIT SHOP DRAWINGS FOR APPROVAL PRIOR TO FABRICATION.
B. ALL STUDS USED FOR EXTERIOR WALL FRAMING SHALL BE 6" STEEL STUDS, MINIMUM 24 GAGE AT 16" O.C. MAXIMUM, UNLESS NOTED OTHERWISE ON PLAN.
C. ALL STUD MEMBERS AND THEIR CONNECTIONS SHALL BE DESIGNED BY THE FABRICATOR FOR A HORRIZONTAL WIND LOAD OF 35 PSF (INWARD OR OUTWARD)
D. DEFLECTION LIMIT = L/240 (EXCEPT L/360 AT INTERIOR PLASTER APPLICATION.
E. OPENINGS 10'-0" OR LESS IN WIDTH SHALL BE FRAMED WITH LIGHT GAGE FRAMING MEMBERS. HEADS OF OPENINGS SHALL CONSIST OF STUD OR JOIST SECTIONS SUFFICIENT TO CARRY THE WEIGHT OF THE WALL ABOVE. JAMB SECTIONS SHALL CONSIST OF HEAVIER GAGE STUDS, MULTIPLE STUDS, OR BOTH, AS REQUIRED TO CARRY THE WIND LOAD OF THE ADJACENT OPENING.
F. ALL FIELD CUTTING TO BE PERFORMED WITH A SAW.
G. WELD SIZE TO BE 3/32" WITH AWS TYPE 6013 OR 7014 ROD.
H. TRACKS TO BE SECURELY ANCHORED TO SUPPORTING STRUCTURE WITH WELD AT EACH SIDE OF TRACKS.
I. PROVIDE CONTINUOUS HORRIZONTAL BRIDGING AT 4'-0" O.C. MAXIMUM FOR WALLS.
J. PROVIDE DOUBLE STUDS UNDER BEAM AND LINTEL BEARING, UNLESS NOTED OTHERWISE.
K. BRIDGING FOR ROOF JOISTS SHALL BE AT 8'-0" O.C. MAXIMUM BETWEEN SUPPORTS.

I. MASONRY
1. MATERIALS:
A. CONCRETE BLOCK: ASTM C90 (HOLLOW), ASTM C145 (SOLID).
B. MORTAR: ASTM C270 TYPE S, MINIMUM COMPRESSIVE STRENGTH: 1800 PSI (PROPERTY SPECIFICATIONS)
C. BOND BEAM AND CORE FILL: ASTM C476, COARSE TYPE.
D. JOINT REINFORCING: MILL GALVANIZED FINISH, 9 GAGE MINIMUM SIDE WIRES AND CROSS WIRES.
E. BAR REINFORCING: ASTM A615, GRADE 60.
2. REINFORCED MASONRY, WHERE VERTICAL BARS ARE TO BE GROUTED INTO CORES, THE FOLLOWING REQUIREMENTS APPLY:
A. PROVIDE DOWELS FROM FOOTING, SAME SIZE AND SPACING AS WALL BARS. LAP 12 INCHES MINIMUM WITH WALL BAR. EMBED INTO FOOTING 9 INCHES.
B. PROVIDE CONTINUOUS VERTICAL CAVITY, AT LEAST 2" X 3" IN SIZE, FREE OF MORTAR DROPPINGS.
C. PROVIDE REBAR ALIGNMENT DEVICES AT A MINIMUM SPACING OF 98 BAR DIAMETERS (MINIMUM OF 2 PER BAR).
D. AT SPLICES IN VERTICAL BARS, PROVIDE MECHANICAL COUPLERS OR LAP OF 48 TIMES THE DIAMETER.
E. ALL REINFORCEMENT MUST BE INSTALLED AND SECURELY ANCHORED IN PLACE PRIOR TO PLACEMENT OF GROUT.
F. MAXIMUM HEIGHT OF GROUT LIFT = 4'-0".
3. MISCELLANEOUS:
A. FILL CORE SOLID AROUND ANCHOR BOLTS.
B. PROVIDE 100% SOLID BLOCKS OR SOLIDLY FITTED HOLLOW BLOCKS AT ALL EXPANSION BOLT LOCATIONS.
C. SET WELD PLATES IN BOND BEAMS AFTER THE GROUT IS PLACED, BUT WHILE IT IS STILL PLASTIC.
D. HOLLOW MASONRY UNITS TO BE LAID WITH FULL MORTAR COVERAGE ON HORIZONTAL AND VERTICAL FACE SHELLS. WEBS SHALL ALSO BE BEDDED IN ALL COURSES OF PIERS, COLUMNS, AND PILASTERS; AND THE STARTING COURSE ON FOOTINGS, AND WHEN ADJACENT TO CELLS OR CAVITIES TO BE REINFORCED OR FILLED WITH CONCRETE OR GROUT. SOLID UNITS TO BE LAID WITH FUL HEAD AND BED JOINTS.
E. PROVIDE JOINT REINFORCING AT 16 INCHES, UNLESS NOTED OTHERWISE.
F. LAP JOINT REINFORCING 6 INCHES FOR STANDARD, 15 INCHES FOR HEAVY WEIGHT.
G. WHERE MASONRY UNITS ARE USED ABOVE HOLLOW UNITS OF A DIFFERENT THICKNESS, PROVIDE A CONTINUOUS COURSE OF 100% SOLID MASONRY AT LEAST 8 INCHES HIGH BELOW TRANSITION.

J. LINTELS:
1. PROVIDE LINTELS OVER ALL OPENINGS IN WALLS. REFER TO ARCHITECTURAL AND HVAC DRAWINGS FOR LOCATION, NUMBER, AND SIZES OF OPENINGS AND LINTEL MATERIAL AND SIZE.

K. STRUCTURAL LUMBER:
1. A. SOUTHERN PINE #2:

SIZE	Fb	Ft	Fv	Fcl	Fc	E
2x4	1500	825	90	565	1650	1,600,000
2x6	1250	725	90	565	1600	1,600,000
2x8	1200	650	90	565	1550	1,600,000
2x10	1050	600	90	565	1500	1,600,000
2x12	975	550	90	565	1450	1,600,000

B. PLYWOOD: C-C PLUGGED, STRUCTURAL II, EXTERIOR GLUE. FOR ROOFS AND WALLS PANEL IDENTIFICATION INDEX 24/0 - 1/2 INCH OR 16/0 - 1/2 INCH (WITH PLYWOOD CLIPS)
2. SPECIFICATIONS: UNLESS SPECIFICALLY SHOWN OTHERWISE, DESIGN, FABRICATION, AND ERCTION SHALL BE GOVERNED BY THE LATEST REVISIONS OF:
A. NATIONAL DESIGN SPECIFICATION FOR STRESS-GRADE LUMBER AND ITS FASTENINGS.
B. U.S. PRODUCT STANDARD PS-1 FOR SOFTWOOD PLYWOOD - CONSTRUCTION AND INDUSTRIAL.
3. CONNECTIONS:
A. JOIST TO BEAMS - 16 GAGE GALVANIZED STANDARD JOIST HANGERS, UNLESS NOTED OTHERWISE.
B. PLYWOOD TO ROOF TRUSSES OR RAFTERS - NAILED - USE 6d RING SHANK NAILS AT 6 INCHES O.C. AT PANEL EDGES AND 12 INCHES O.C. AT INTERMEDIATE SUPPORTS. PROVIDE PLYWOOD CLIPS AT MIDSPAN OF PLYWOOD BETWEEN SUPPORTS.
4. MISCELANEOUS:
A. USE ONE LINE OF SOLID BLOCKING OR CROSS BRIDGING AT 8'-0" O.C. MAX FOR ALL JOISTS AND RAFTERS, USE SOLID BLOCKING AT JIOST AND RAFTER BEARING.
B. USE SOLID BLOCKING AT MID-HEIGHT FOR ALL EXTERIOR STUD WALLS AND INTERIOR BEARING PARTITIONS.
C. USE DOUBLE STUDS UNDER BEAM AND LINTEL BEARING, UNLESS NOTED OTHERWISE.

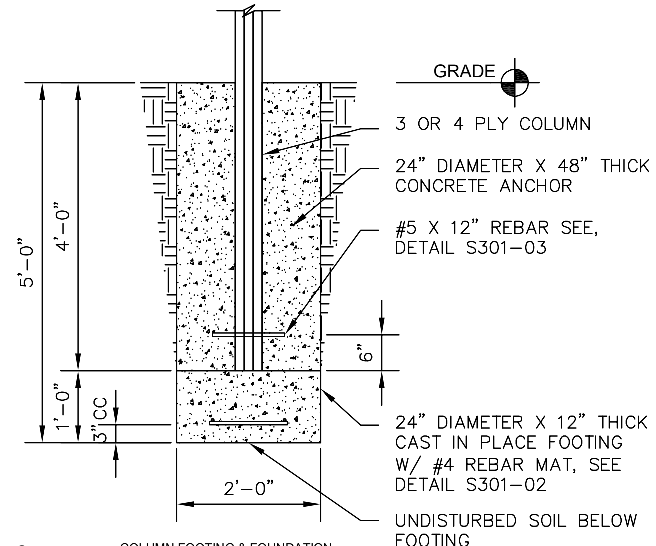
L. PREFABRICATED WOOD TRUSSES:
1. MATERIALS:
A. LUMBER: SOUTHERN PINE #2: Fb = 1500 (REPETATIVE USE) PSI, Ft = 675 PSI, Fc (PARALELL TO GRAIN) = 1150 PSI, Fc (PERPENDICULAR TO GRAIN) = 565 PSI, E = 1600 KSI MAX., M.C. = 15%.
B. METAL CONNECTOR PLATES: GALVANIZED SHEET STEEL ASTM A653/A653M-11 GRADE A. COATING CLASS G60 PER ASTM A653/A653M-11. MANUFACTURED WITH HOLES, PLUGS, TEETH, OR PRONGS UNIFORMLY SPACED AND FORMED.
2. DESIGN CRITERIA:
A. LOADING:

1. TOP CHORD LIVE LOAD:	25 PSF
2. TOP CHORD DEAD LOAD:	11 PSF
3. BOTTOM CHORD DEAD LOAD:	7 PSF
4. NET WIND UPLIFT:	8 PSF

B. DESIGN OF MEMBERS AND CONNECTIONS IS TO BE BY A PROFESSIONAL ENGINEER, REGISTERED IN OHIO. EXPERIENCED IN SIMILAR DESIGN, RETAINED BY THE MANUFACTURER.
C. SHOP DRAWINGS SHALL EXHIBIT THE SEAL OF THE ENGINEER RESPONSIBLE FOR THE TRUSS DESIGN. IN ADDITION, DESIGN CALCULATIONS FOR THESE TRUSSES SHALL BE SUBMITTED TO THE ARCHITECT FOR REVIEW.
D. MEMBER SIZES SHOWN ARE MINIMUM SIZES.
E. MAXIMUM LIVE LOAD DEFLECTION IS TO BE L/360.
F. MAXIMUM TOTAL LOAD DEFLECTION IS TO BE L/240.
3. MISCELANEOUS:
A. BOLT TOP CHORDS OF ALL MULTIPLE MEMBER TRUSSES TOGETHER WITH 3/4" DIAMETER BOLTS AT 4'-0" O.C.. BOLT WEB MEMBERS TOGETHER WITH 34"
E. MAXIMUM LIVE LOAD DEFLECTION IS TO BE L/360.
F. MAXIMUM TOTAL LOAD DEFLECTION IS TO BE L/240.
3. MISCELANEOUS:
A. BOLT TOP CHORDS OF ALL MULTIPLE MEMBER TRUSSES TOGETHER WITH 3/4" DIAMETER BOLTS AT 4'-0" O.C.. BOLT WEB MEMBERS TOGETHER WITH 34" DIAMETER BOLTS AT 2'-0" O.C. AT CONCENTRATED LOADS.
B. VWERIFY ALL DIMENSIONS, ELEVATIONS, AND SLOPES PRIOR TO MANUFACTURING. REPORT ANY DISCREPENCIES IMMEDIATELY TO THE ARCHITECT.
C. WOOD TRUSSES SHALL BE DESIGNED AND FABRICATED TO CONFORM TO THE GEOMETRIES SHOWN ON THE DRAWINGS. WEB CONFIGURATIONS ARE TO BE DETAILES AS REQUIRED BY THE DESIGNER/FABRICATOR.
D. IN AREAS WHERE TOP CHORD OF TRUSSES DO NOT RECIEVE PLYWOOD SHEETING, PROVIDE 1x4 CONTINUOUS BRIDGING PERPENDICULAR TO TOP CHORDS AND SPACED AT 3'-0" O.C..
E. TRUSS FABRICATOR SHALL SUBMIT COPIES OF THE FINAL APPROVED FABRICATION DRAWINGS TO THE OHIO DEPARTMENT OF INDUSTRIAL RELATIONS, DIVISION OF FACTORY AND BUILDING INSPECTION, PRIOR TO FABRICATION AND ERECTION.

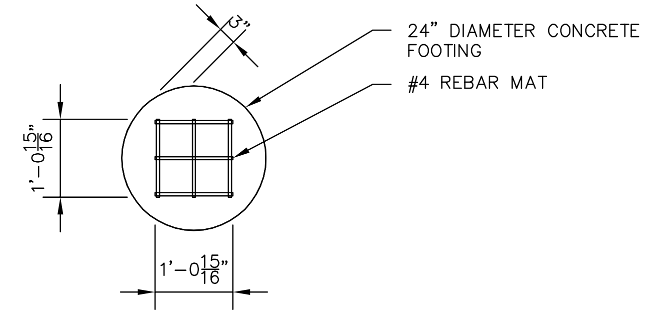
M. ABBREVIATIONS:
B = BOTTOM
CC = CLEAR COVER
C.M.U. = CONCRETE MASONRY UNIT
E.E. = EACH END
E.F. = EACH FACE
E.L.F. = EQUIVALENT LATERAL FORCE
E.W. = EACH WAY
J.B. = JOIST BEARING ELEVATION
O.C. = ON CENTER
S.O.G. = SLAB ON GRADE
T = TOP
T.B. = TRUSS BEARING ELEVATION
T.O.F. = TOP OF FOOTING ELEVATION
T.O.P. = TOP OF PIER
T.O.S. = TOP OF SLAB ELEVATION
T.O.W. = TOP OF WALL ELEVATION
TYP. = TYPICAL
U.N.O. = UNLESS NOTED OTHERWISE
WWF = WELDED WIRE FABRIC

FOOTING EXCAVATIONS MUST BE APPROVED BY A SOILS ENGINEER PRIOR TO CONCRETE PLACEMENT. SEE DETAIL S301-06 FOR ALT. FOUNDATION.



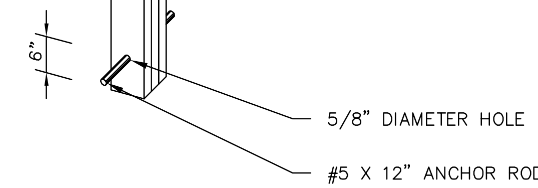
S301-01 COLUMN FOOTING & FOUNDATION
SCALE: N.T.S.

FOOTING EXCAVATIONS MUST BE APPROVED BY A SOILS ENGINEER PRIOR TO CONCRETE PLACEMENT. SEE DETAIL S301-06 FOR ALT. FOUNDATION.



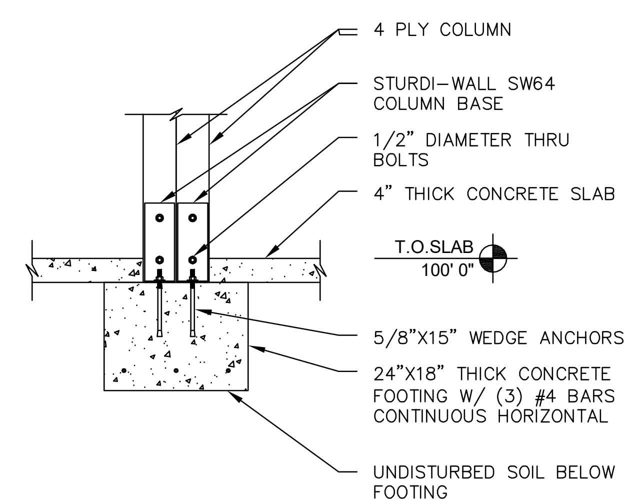
S301-02 REBAR MAT
SCALE: N.T.S.

FOOTING EXCAVATIONS MUST BE APPROVED BY A SOILS ENGINEER PRIOR TO CONCRETE PLACEMENT. SEE DETAIL S301-06 FOR ALT. FOUNDATION.



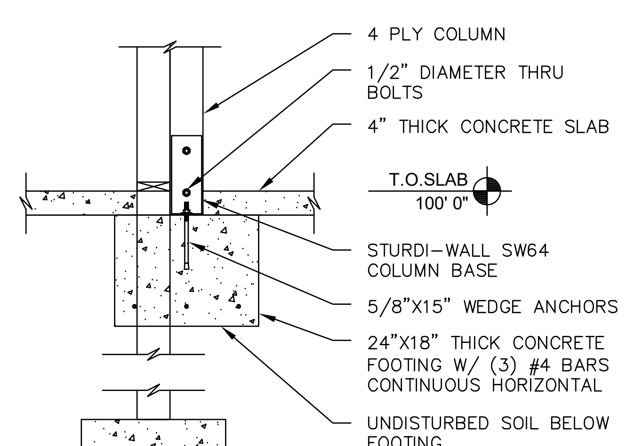
S301-03 COLUMN ANCHOR ROD
SCALE: N.T.S.

FOOTING EXCAVATIONS MUST BE APPROVED BY A SOILS ENGINEER PRIOR TO CONCRETE PLACEMENT. SEE DETAIL S301-06 FOR ALT. FOUNDATION.



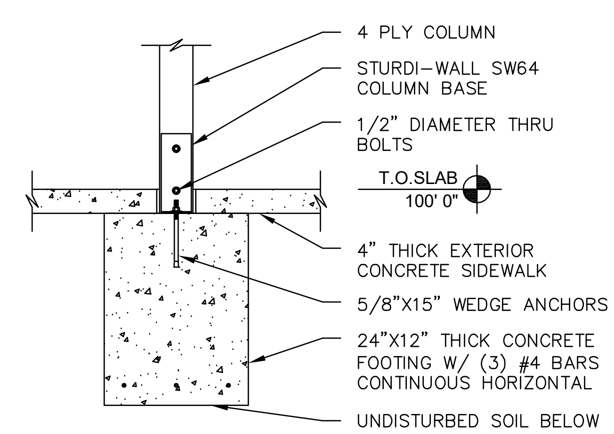
S301-04 COLUMN LINE D 24" X 12" TALL FOOTING
SCALE: N.T.S.

FOOTING EXCAVATIONS MUST BE APPROVED BY A SOILS ENGINEER PRIOR TO CONCRETE PLACEMENT. SEE DETAIL S301-06 FOR ALT. FOUNDATION.



S301-05 COLUMN LINE D 24" X 18.5" TALL FOOTING
SCALE: N.T.S.

FOOTING EXCAVATIONS MUST BE APPROVED BY A SOILS ENGINEER PRIOR TO CONCRETE PLACEMENT. SEE DETAIL S301-06 FOR ALT. FOUNDATION.



S301-06 ALTERNATE EXTERIOR PERIMETER COLUMN CONTINUOUS TRENCH FOUNDATION
SCALE: N.T.S.

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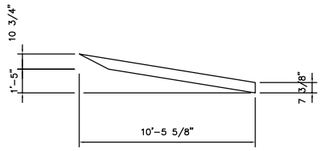
STATE OF OHIO
JONATHAN T. MILLER
E-68072
REGISTERED
PROFESSIONAL ENGINEER

SUBMITTAL DATE
12/23/20

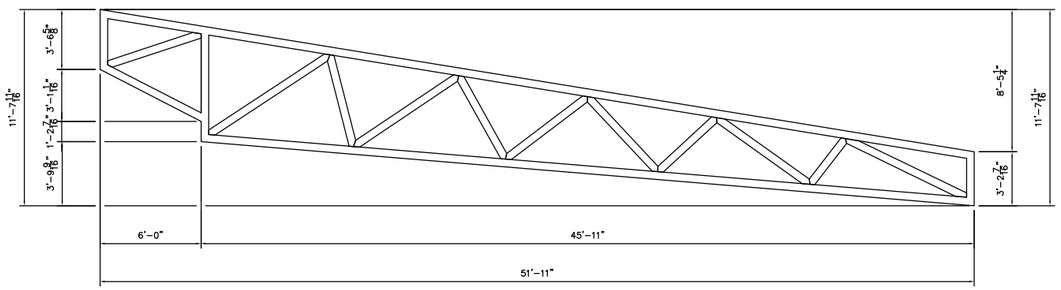
A201265

STRUCTURAL NOTES

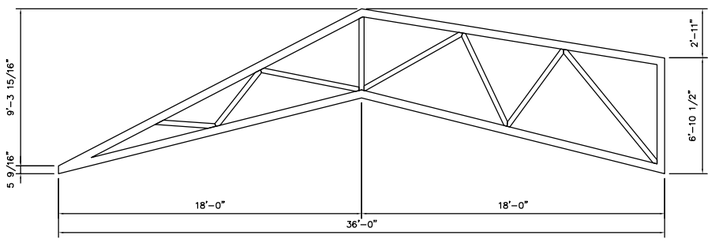
S301



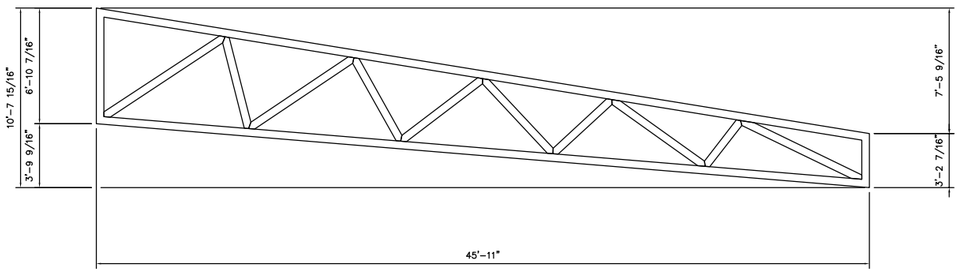
S302-01 2 PLY 1-1/2" X 7-1/4" LVL RAFTER
SCALE: 1/4" = 1'0"



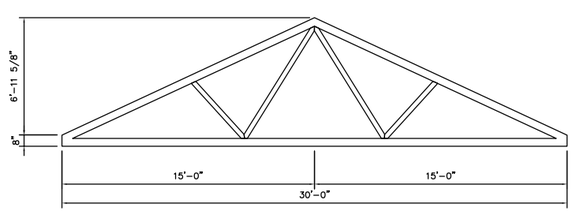
S302-02 PRE-ENGINEERED TRUSS A
SCALE: 1/4" = 1'0"



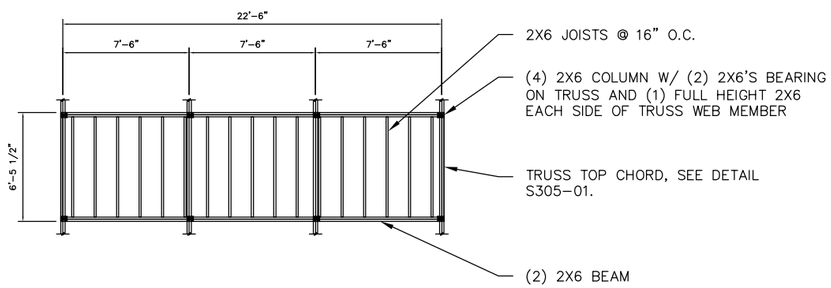
S302-03 PRE-ENGINEERED TRUSS B
SCALE: 1/4" = 1'0"



S302-04 PRE-ENGINEERED TRUSS C
SCALE: 1/4" = 1'0"



S302-05 PRE-ENGINEERED TRUSS D
SCALE: 1/4" = 1'0"



S302-06 MECHANICAL PLATFORM
SCALE: 1/4" = 1'0"

GENERAL NOTES:

K. STRUCTURAL LUMBER:
1. A. SOUTHERN PINE #2:

SIZE	Fb	Ft	Fv	Fcl	Fc	E
2x4	1500	825	90	565	1650	1,600,000
2x6	1250	725	90	565	1600	1,600,000
2x8	1200	650	90	565	1550	1,600,000
2x10	1050	600	90	565	1500	1,600,000
2x12	975	550	90	565	1450	1,600,000

B. PLYWOOD: C-C PLUGGED, STRUCTURAL II, EXTERIOR GLUE. FOR ROOFS AND WALLS PANEL IDENTIFICATION INDEX 24/0 - 1/2 INCH OR 16/0 - 1/2 INCH (WITH PLYWOOD CLIPS)
2. SPECIFICATIONS: UNLESS SPECIFICALLY SHOWN OTHERWISE, DESIGN, FABRICATION, AND ERCTION SHALL BE GOVERNED BY THE LATEST REVISIONS OF:
A. NATIONAL DESIGN SPECIFICATION FOR STRESS-GRADE LUMBER AND ITS FASTENINGS.
B. U.S. PRODUCT STANDARD PS-1 FOR SOFTWOOD PLYWOOD - CONSTRUCTION AND INDUSTRIAL
3. CONNECTIONS:
A. JOIST TO BEAMS - 16 GAGE GALVANIZED STANDARD JOIST HANGERS, UNLESS NOTED OTHERWISE.
B. PLYWOOD TO ROOF TRUSSES OR RAFTERS - NAILED - USE 6d RING SHANK NAILS AT 6 INCHES O.C. AT PANEL EDGES AND 12 INCHES O.C. AT INTERMEDIATE SUPPORTS. PROVIDE PLYWOOD CLIPS AT MIDSPAN OF PLYWOOD BETWEEN SUPPORTS.
4. MISCELANEOUS:
A. USE ONE LINE OF SOLID BLOCKING OR CROSS BRIDGING AT 8'-0" O.C. MAX FOR ALL JOISTS AND RAFTERS, USE SOLID BLOCKING AT JIOST AND RAFTER BEARING.
B. USE SOLID BLOCKING AT MID-HEIGHT FOR ALL EXTERIOR STUD WALLS AND INTERIOR BEARING PARTITIONS.
C. USE DOUBLE STUDS UNDER BEAM AND LINTEL BEARING, UNLESS NOTED OTHERWISE.

L. PREFABRICATED WOOD TRUSSES:
1. MATERIALS:
A. LUMBER: SOUTHERN PINE #2: Fb = 1500 (REPETATIVE USE) PSI, Ft = 675 PSI, Fc (PARALELL TO GRAIN) = 1150 PSI, Fc (PERPENDICULAR TO GRAIN) = 565 PSI, E = 1600 KSI MAX., M.C. = 15%.
B. METAL CONNECTOR PLATES: GALVANIZED SHEET STEEL ASTM A653/A653M-11 GRADE A, COATING CLASS G60 PER ASTM A653/A653M-11. MANUFACTURED WITH HOLES, PLUGS, TEETH, OR PRONGS UNIFORMLY SPACED AND FORMED.
2. DESIGN CRITERIA:
A. LOADING:
1. TOP CHORD LIVE LOAD: 25 PSF
2. TOP CHORD DEAD LOAD: 11 PSF
3. BOTTOM CHORD DEAD LOAD: 7 PSF
4. NET WIND UPLIFT: 8 PSF
B. DESIGN OF MEMBERS AND CONNECTIONS IS TO BE BY A PROFESSIONAL ENGINEER, REGISTERED IN OHIO. EXPERIENCED IN SIMILAR DESIGN, RETAINED BY THE MANUFACTURER.
C. SHOP DRAWINGS SHALL EXHIBIT THE SEAL OF THE ENGINEER RESPONSIBLE FOR THE TRUSS DESIGN. IN ADDITION, DESIGN CALCULATIONS FOR THESE TRUSSES SHALL BE SUBMITTED TO THE ARCHITECT FOR REVIEW.
D. MEMBER SIZES SHOWN ARE MINIMUM SIZES.
E. MAXIMUM LIVE LOAD DEFLECTION IS TO BE L/360.
F. MAXIMUM TOTAL LOAD DEFLECTION IS TO BE L/240.
3. MISCELANEOUS:
A. BOLT TOP CHORDS OF ALL MULTIPLE MEMBER TRUSSES TOGETHER WITH 3/4" DIAMETER BOLTS AT 4'-0" O.C.. BOLT WEB MEMBERS TOGETHER WITH 3/4"
E. MAXIMUM LIVE LOAD DEFLECTION IS TO BE L/360.
F. MAXIMUM TOTAL LOAD DEFLECTION IS TO BE L/240.
3. MISCELANEOUS:
A. BOLT TOP CHORDS OF ALL MULTIPLE MEMBER TRUSSES TOGETHER WITH 3/4" DIAMETER BOLTS AT 4'-0" O.C.. BOLT WEB MEMBERS TOGETHER WITH 3/4" DIAMETER BOLTS AT 2'-0" O.C. AT CONCENTRATED LOADS.
B. VWERIFY ALL DIMENSIONS, ELEVATIONS, AND SLOPES PRIOR TO MANUFACTURING. REPORT ANY DISCREPENCIES IMMEDIATELY TO THE ARCHITECT.
C. WOOD TRUSSES SHALL BE DESIGNED AND FABRICATED TO CONFORM TO THE GEOMETRIES SHOWN ON THE DRAWINGS. WEB CONFIGURATIONS ARE TO BE DETAILES AS REQUIRED BY THE DESIGNER/FABRICATOR.
D. IN AREAS WHERE TOP CHORD OF TRUSSES DO NOT RECIEVE PLYWOOD SHEETING, PROVIDE 1x4 CONTINUOUS BRIDGING PERPENDICULAR TO TOP CHORDS AND SPACED AT 3'-0" O.C..
E. TRUSS FABRICATOR SHALL SUBMIT COPIES OF THE FINAL APPROVED FABRICATION DRAWINGS TO THE OHIO DEPARTMENT OF INDUSTRIAL RELATIONS, DIVISION OF FACTORY AND BUILDING INSPECTION, PRIOR TO FABRICATION AND ERECTION.

SHEET NOTES:

KEYED NOTES:

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SUBMITTAL DATE
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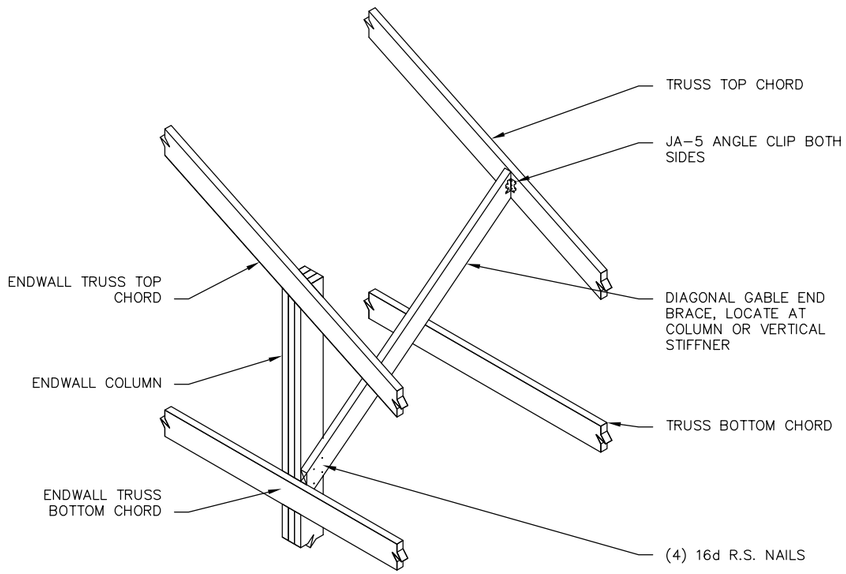
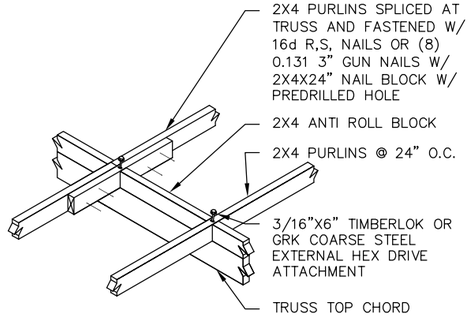
A201265

TRUSS AND RAFTER PROFILES

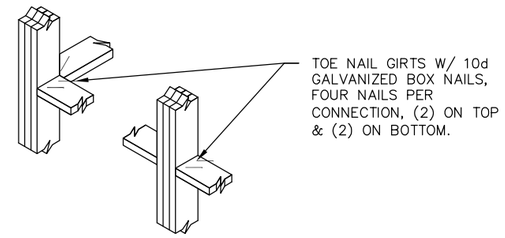
S302



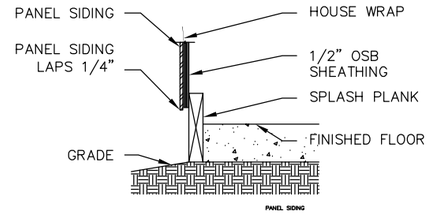
S303-01 DIAGONAL GABLE BRACE
SCALE: N.T.S.



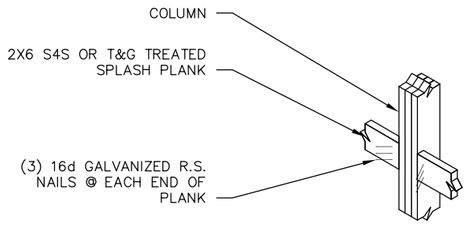
S303-02 BYPASS PURLIN
SCALE: N.T.S.



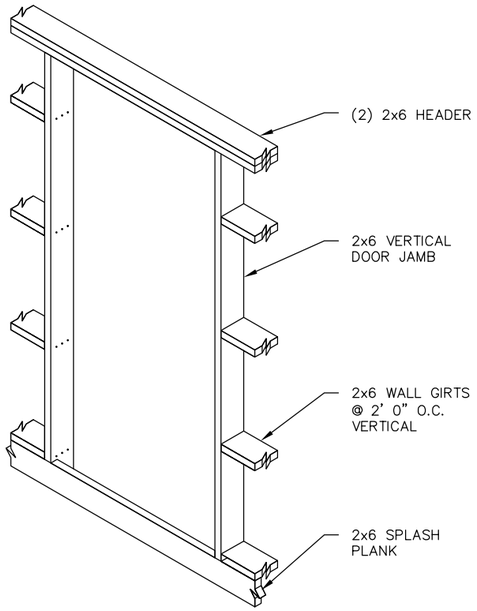
S304-03 INSET GIRT
SCALE: N.T.S.



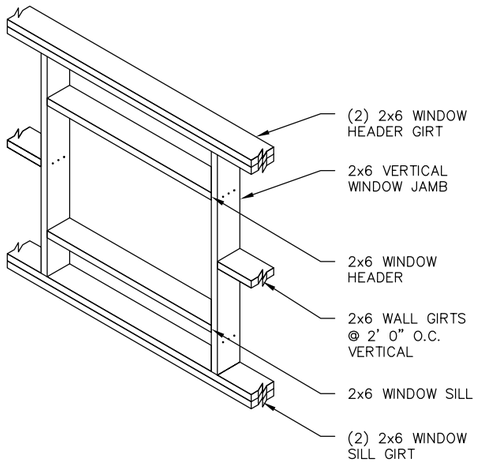
S303-04 PANEL SIDING
SCALE: N.T.S.



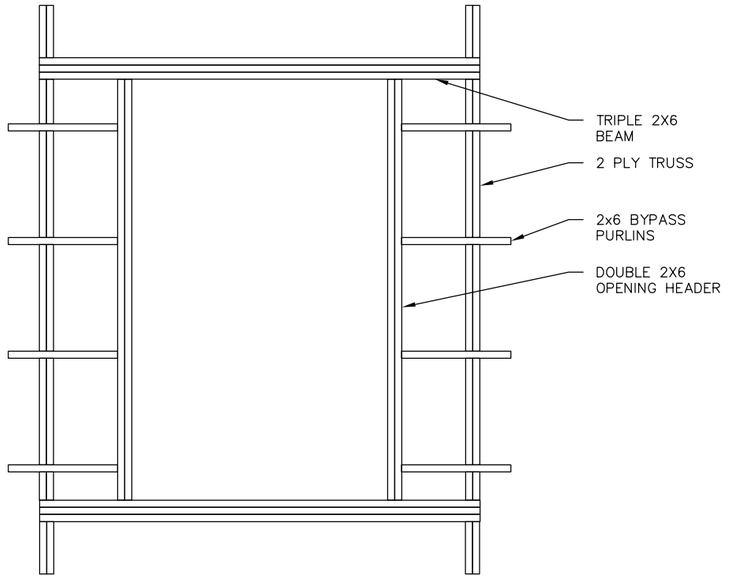
S303-05 INSET SPLASH PLANK
SCALE: N.T.S.



S303-06 DOOR FRAMING DETAIL
SCALE: N.T.S.



S303-07 WINDOW FRAMING DETAIL
SCALE: N.T.S.



S303-08 DORMER SUPPORT FRAMING
SCALE: N.T.S.

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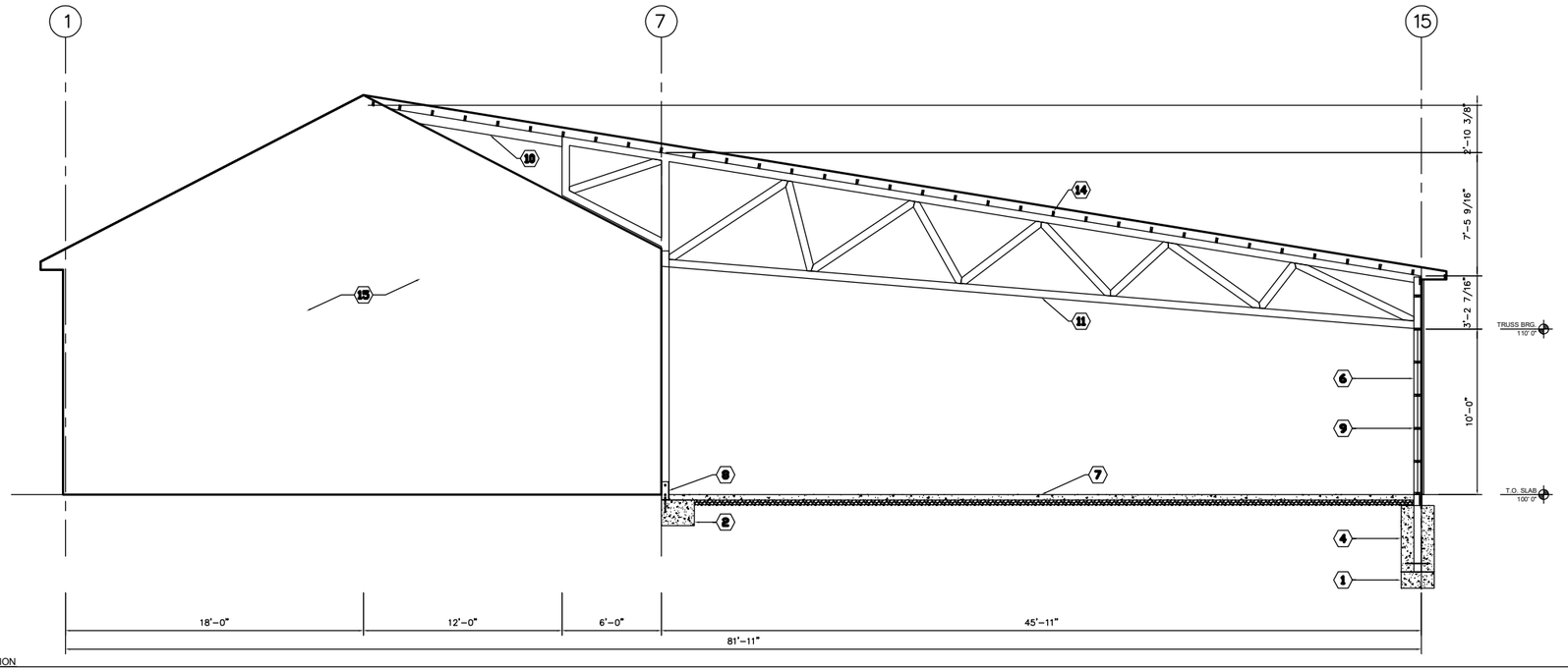
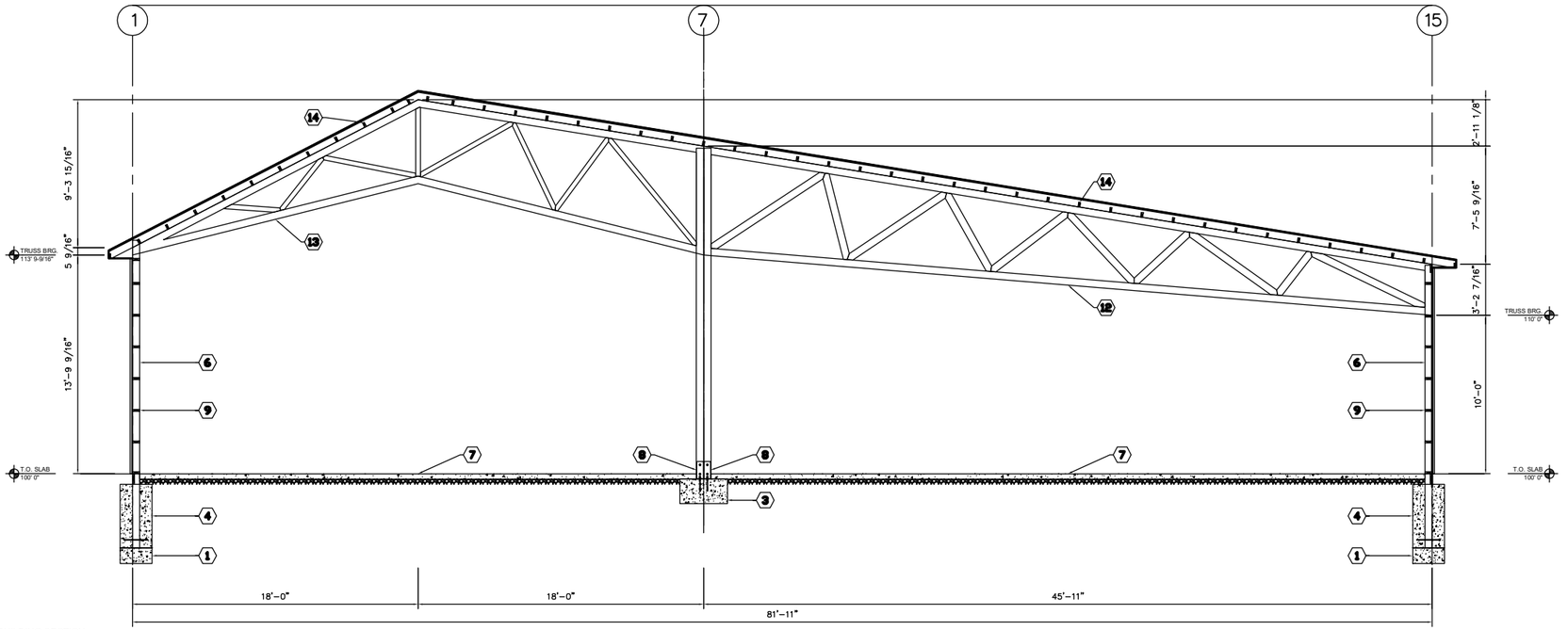
DETAILS

S303



S305-01 BUILDING SECTION
SCALE: 1/4" = 1'-0"



S305-02 BUILDING SECTION
SCALE: 1/4" = 1'-0"

GENERAL NOTES:

K. STRUCTURAL LUMBER:

1. A. SOUTHERN PINE #2:

SIZE	Fb	Ft	Fv	Fcl	Fc	E
2x4	1500	825	90	565	1650	1,600,000
2x6	1250	725	90	565	1600	1,600,000
2x8	1200	650	90	565	1550	1,600,000
2x10	1050	600	90	565	1500	1,600,000
2x12	975	550	90	565	1450	1,600,000

B. PLYWOOD: C-C PLUGGED, STRUCTURAL II, EXTERIOR GLUE. FOR ROOFS AND WALLS PANEL IDENTIFICATION INDEX 24/0 - 1/2 INCH OR 16/0 - 1/2 INCH (WITH PLYWOOD CLIPS)

2. SPECIFICATIONS: UNLESS SPECIFICALLY SHOWN OTHERWISE, DESIGN, FABRICATION, AND ERCTION SHALL BE GOVERNED BY THE LATEST REVISIONS OF:
   A. NATIONAL DESIGN SPECIFICATION FOR STRESS-GRADE LUMBER AND ITS FASTENINGS.
   B. U.S. PRODUCT STANDARD PS-1 FOR SOFTWOOD PLYWOOD - CONSTRUCTION AND INDUSTRIAL
3. CONNECTIONS:
   A. JOIST TO BEAMS - 16 GAGE GALVANIZED STANDARD JOIST HANGERS, UNLESS NOTED OTHERWISE.
   B. PLYWOOD TO ROOF TRUSSES OR RAFTERS - NAILED - USE 6d RING SHANK NAILS AT 6 INCHES O.C. AT PANEL EDGES AND 12 INCHES O.C. AT INTERMEDIATE SUPPORTS. PROVIDE PLYWOOD CLIPS AT MIDSPAN OF PLYWOOD BETWEEN SUPPORTS.
4. MISCELANEOUS:
   A. USE ONE LINE OF SOLID BLOCKING OR CROSS BRIDGING AT 8'-0" O.C. MAX FOR ALL JOISTS AND RAFTERS, USE SOLID BLOCKING AT JIOST AND RAFTER BEARING.
   B. USE SOLID BLOCKING AT MID-HEIGHT FOR ALL EXTERIOR STUD WALLS AND INTERIOR BEARING PARTITIONS.
   C. USE DOUBLE STUDS UNDER BEAM AND LINTEL BEARING, UNLESS NOTED OTHERWISE.

L. PREFABRICATED WOOD TRUSSES:

1. MATERIALS:
   A. LUMBER: SOUTHERN PINE #2: Fb = 1500 (REPETATIVE USE) PSI, Ft = 675 PSI, Fc (PARALELL TO GRAIN) = 1150 PSI, Fc (PERPENDICULAR TO GRAIN) = 565 PSI, E = 1600 KSI MAX., M.C. = 15%.
   B. METAL CONNECTOR PLATES: GALVANIZED SHEET STEEL ASTM A653/A653M-11 GRADE A. COATING CLASS G60 PER ASTM A653/A653M-11. MANUFACTURED WITH HOLES, PLUGS, TEETH, OR PRONGS UNIFORMLY SPACED AND FORMED.
2. DESIGN CRITERIA:
   A. LOADING:
      1. TOP CHORD LIVE LOAD: 25 PSF
      2. TOP CHORD DEAD LOAD: 11 PSF
      3. BOTTOM CHORD DEAD LOAD: 7 PSF
      4. NET WIND UPLIFT: 8 PSF
   B. DESIGN OF MEMBERS AND CONNECTIONS IS TO BE BY A PROFESSIONAL ENGINEER, REGISTERED IN OHIO. EXPERIENCED IN SIMILAR DESIGN, RETAINED BY THE MANUFACTURER.
   C. SHOP DRAWINGS SHALL EXHIBIT THE SEAL OF THE ENGINEER RESPONSIBLE FOR THE TRUSS DESIGN. IN ADDITION, DESIGN CALCULATIONS FOR THESE TRUSSES SHALL BE SUBMITTED TO THE ARCHITECT FOR REVIEW.
   D. MEMBER SIZES SHOWN ARE MINIMUM SIZES.
   E. MAXIMUM LIVE LOAD DEFLECTION IS TO BE L/360.
   F. MAXIMUM TOTAL LOAD DEFLECTION IS TO BE L/240.
3. MISCELANEOUS:

A. BOLT TOP CHORDS OF ALL MULTIPLE MEMBER TRUSSES TOGETHER WITH 3/4" DIAMETER BOLTS AT 4'-0" O.C.. BOLT WEB MEMBERS TOGETHER WITH 3/4"
   E. MAXIMUM LIVE LOAD DEFLECTION IS TO BE L/360.
   F. MAXIMUM TOTAL LOAD DEFLECTION IS TO BE L/240.

3. MISCELANEOUS:
   A. BOLT TOP CHORDS OF ALL MULTIPLE MEMBER TRUSSES TOGETHER WITH 3/4" DIAMETER BOLTS AT 4'-0" O.C.. BOLT WEB MEMBERS TOGETHER WITH 3/4" DIAMETER BOLTS AT 2'-0" O.C. AT CONCENTRATED LOADS.
   B. VWERIFY ALL DIMENSIONS, ELEVATIONS, AND SLOPES PRIOR TO MANUFACTURING. REPORT ANY DISCREPENCIES IMMEDIATELY TO THE ARCHITECT.
   C. WOOD TRUSSES SHALL BE DESIGNED AND FABRICATED TO CONFORM TO THE GEOMETRIES SHOWN ON THE DRAWINGS. WEB CONFIGURATIONS ARE TO BE DETAILES AS REQUIRED BY THE DESIGNER/FABRICATOR.
   D. IN AREAS WHERE TOP CHORD OF TRUSSES DO NOT RECIEVE PLYWOOD SHEETING, PROVIDE 1x4 CONTINUOUS BRIDGING PERPENDICULAR TO TOP CHORDS AND SPACED AT 3'-0" O.C..
   E. TRUSS FABRICATOR SHALL SUBMIT COPIES OF THE FINAL APPROVED FABRICATION DRAWINGS TO THE OHIO DEPARTMENT OF INDUSTRIAL RELATIONS, DIVISION OF FACTORY AND BUILDING INSPECTION, PRIOR TO FABRICATION AND ERECTION.

SHEET NOTES:

KEYED NOTES:

1. 12" THICK X 24" DIAMETER CONCRETE , SEE DETAIL: S301-01
2. 18-1/2" THICK X 24" WIDE CONCRETE FOOTING W/ (2) #4 BARS CONTINUOUS HORIZONTAL, SEE DETAIL:S301-05.
3. 12" THICK X 24" WIDE CONCRETE FOOTING W/ (2) #4 BARS CONTINUOUS HORIZONTAL, SEE DETAIL:S301-04.
4. 24" DIAMETER X 48" THICK CONCRETE ANCHOR, SEE DETAIL S301-01.
5. 3 PLY 2X6 LAMINATED COLUMN WITH #4 BAR ANCHOR ROD, SEE DETAIL S301-03.
6. 4 PLY 2X6 LAMINATED COLUMN WITH #4 BAR ANCHOR ROD, SEE DETAIL S301-03.
7. 4" THICK CONCRETE SLAB w/ 6x6 W2.9 x W2.9 WWF ON 6 MIL POLY VAPOR BARRIER ON 4" COMPACTED GRANNULAR FILL.
8. STURDI-WALL SW64 COLUMN BASE, SEE DETAIL: S301-05.
9. 2x6 INSET GIRTS AT 24" O.C. VERTICAL, SEE DETAIL:
10. 2 PLY 1-1/2 X 7-1/4 LVL RAFTERS @ 7'6" O.C. SEE DETAIL: S302-01.
11. PRE-ENGINEERED A TRUSSES @ 7'6" O.C. SEE DETAIL: S302-02
12. PRE-ENGINEERED B TRUSSES @ 7'6" O.C. SEE DETAIL: S302-03
13. PRE-ENGINEERED C TRUSSES @ 7'6" O.C. SEE DETAIL: S302-04
14. 2x4 BYPASS PURLINS AT 24" O.C. UP THE SLOPE.
15. EXISTING STRUCTURE..

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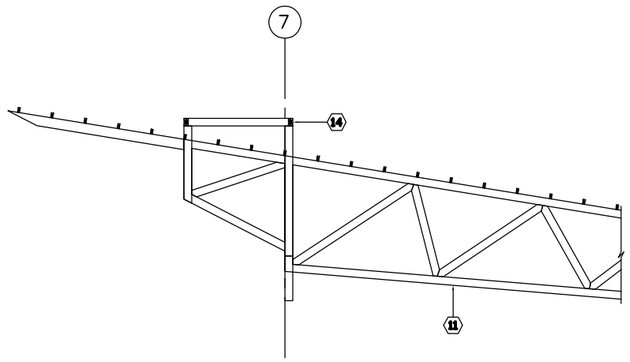
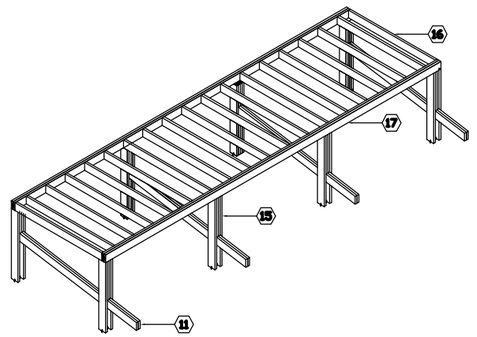
TRUSS ELEVATION

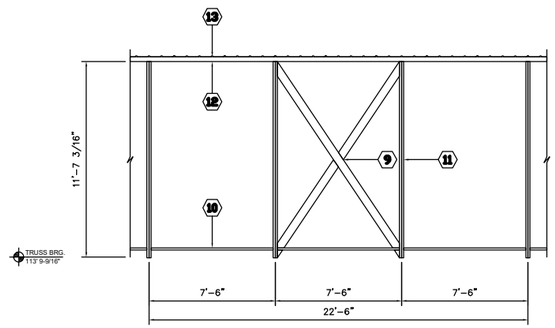
S304



S305-01 MECHANICAL PLATFORM
SCALE: 1/4" = 1'0"

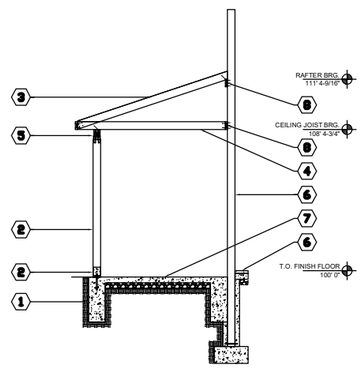


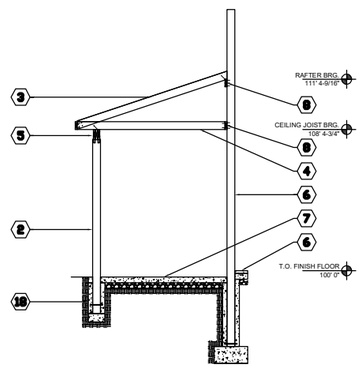
S305-02 MECHANICAL PLATFORM ORTHAGONAL
SCALE: 1/4" = 1'0"



S305-03 TRUSS BRACING
SCALE: 1/4" = 1'0"

FOOTING EXCAVATIONS MUST BE APPROVED BY A SOILS ENGINEER PRIOR TO CONCRETE PLACEMENT. SEE DETAIL S301-06 FOR ALT. FOUNDATION.





S305-04 COVERED WALK FRAMING
SCALE: 1/4" = 1'0"

GENERAL NOTES:

K. STRUCTURAL LUMBER:
1. A. SOUTHERN PINE #2:

SIZE	Fb	Ft	Fv	Fcl	Fc	E
2x4	1500	825	90	565	1650	1,600,000
2x6	1250	725	90	565	1600	1,600,000
2x8	1200	650	90	565	1550	1,600,000
2x10	1050	600	90	565	1500	1,600,000
2x12	975	550	90	565	1450	1,600,000

B. PLYWOOD: C-C PLUGGED, STRUCTURAL II, EXTERIOR GLUE. FOR ROOFS AND WALLS PANEL IDENTIFICATION INDEX 24/0 - 1/2 INCH OR 16/0 - 1/2 INCH (WITH PLYWOOD CLIPS)

2. SPECIFICATIONS: UNLESS SPECIFICALLY SHOWN OTHERWISE, DESIGN, FABRICATION, AND ERCTION SHALL BE GOVERNED BY THE LATEST REVISIONS OF:
A. NATIONAL DESIGN SPECIFICATION FOR STRESS-GRADE LUMBER AND ITS FASTENINGS.
B. U.S. PRODUCT STANDARD PS-1 FOR SOFTWOOD PLYWOOD - CONSTRUCTION AND INDUSTRIAL

3. CONNECTIONS:
A. JOIST TO BEAMS - 16 GAGE GALVANIZED STANDARD JOIST HANGERS, UNLESS NOTED OTHERWISE.
B. PLYWOOD TO ROOF TRUSSES OR RAFTERS - NAILED - USE 6d RING SHANK NAILS AT 6 INCHES O.C. AT PANEL EDGES AND 12 INCHES O.C. AT INTERMEDIATE SUPPORTS. PROVIDE PLYWOOD CLIPS AT MIDSPAN OF PLYWOOD BETWEEN SUPPORTS.

4. MISCELANEOUS:
A. USE ONE LINE OF SOLID BLOCKING OR CROSS BRIDGING AT 8'-0" O.C. MAX FOR ALL JOISTS AND RAFTERS, USE SOLID BLOCKING AT JIOST AND RAFTER BEARING.
B. USE SOLID BLOCKING AT MID-HEIGHT FOR ALL EXTERIOR STUD WALLS AND INTERIOR BEARING PARTITIONS.
C. USE DOUBLE STUDS UNDER BEAM AND LINTEL BEARING, UNLESS NOTED OTHERWISE.

L. PREFABRICATED WOOD TRUSSES:
1. MATERIALS:
A. LUMBER: SOUTHERN PINE #2: Fb = 1500 (REPETATIVE USE) PSI, Ft = 675 PSI, Fc (PARALELL TO GRAIN) = 1150 PSI, Fc (PERPENDICULAR TO GRAIN) = 565 PSI, E = 1600 KSI MAX., M.C. = 15%.
B. METAL CONNECTOR PLATES: GALVANIZED SHEET STEEL ASTM A653/A653M-11 GRADE A, COATING CLASS G60 PER ASTM A653/A653M-11. MANUFACTURED WITH HOLES, PLUGS, TEETH, OR PRONGS UNIFORMLY SPACED AND FORMED.
2. DESIGN CRITERIA:
A. LOADING:
1. TOP CHORD LIVE LOAD: 25 PSF
2. TOP CHORD DEAD LOAD: 11 PSF
3. BOTTOM CHORD DEAD LOAD: 7 PSF
4. NET WIND UPLIFT: 8 PSF
B. DESIGN OF MEMBERS AND CONNECTIONS IS TO BE BY A PROFESSIONAL ENGINEER, REGISTERED IN OHIO. EXPERIENCED IN SIMILAR DESIGN, RETAINED BY THE MANUFACTURER.
C. SHOP DRAWINGS SHALL EXHIBIT THE SEAL OF THE ENGINEER RESPONSIBLE FOR THE TRUSS DESIGN. IN ADDITION, DESIGN CALCULATIONS FOR THESE TRUSSES SHALL BE SUBMITTED TO THE ARCHITECT FOR REVIEW.
D. MEMBER SIZES SHOWN ARE MINIMUM SIZES.
E. MAXIMUM LIVE LOAD DEFLECTION IS TO BE L/360.
F. MAXIMUM TOTAL LOAD DEFLECTION IS TO BE L/240.
3. MISCELANEOUS:
A. BOLT TOP CHORDS OF ALL MULTIPLE MEMBER TRUSSES TOGETHER WITH 3/4" DIAMETER BOLTS AT 4'-0" O.C.. BOLT WEB MEMBERS TOGETHER WITH 3/4"
E. MAXIMUM LIVE LOAD DEFLECTION IS TO BE L/360.
F. MAXIMUM TOTAL LOAD DEFLECTION IS TO BE L/240.
3. MISCELANEOUS:
A. BOLT TOP CHORDS OF ALL MULTIPLE MEMBER TRUSSES TOGETHER WITH 3/4" DIAMETER BOLTS AT 4'-0" O.C.. BOLT WEB MEMBERS TOGETHER WITH 3/4" DIAMETER BOLTS AT 2'-0" O.C. AT CONCENTRATED LOADS.
B. VWERIFY ALL DIMENSIONS, ELEVATIONS, AND SLOPES PRIOR TO MANUFACTURING. REPORT ANY DISCREPENCIES IMMEDIATELY TO THE ARCHITECT.
C. WOOD TRUSSES SHALL BE DESIGNED AND FABRICATED TO CONFORM TO THE GEOMETRIES SHOWN ON THE DRAWINGS. WEB CONFIGURATIONS ARE TO BE DETAILES AS REQUIRED BY THE DESIGNER/FABRICATOR.
D. IN AREAS WHERE TOP CHORD OF TRUSSES DO NOT RECIEVE PLYWOOD SHEETING, PROVIDE 1x4 CONTINUOUS BRIDGING PERPENDICULAR TO TOP CHORDS AND SPACED AT 3'-0" O.C..
E. TRUSS FABRICATOR SHALL SUBMIT COPIES OF THE FINAL APPROVED FABRICATION DRAWINGS TO THE OHIO DEPARTMENT OF INDUSTRIAL RELATIONS, DIVISION OF FACTORY AND BUILDING INSPECTION, PRIOR TO FABRICATION AND ERECTION.

SHEET NOTES:

KEYED NOTES:

1. 12" THICK X 32" DEEP TRENCH TURNDOWN SLAB FOUNDATION.
2. 4X6 PT COLUMN WITH SIMPSON ABU45 COLUMN BASE SEE DETAILS S301-04 & S301-05, SIMILAR.
3. 2X6 RAFTERS @ 24" O.C., ATTACH TO LEDGER W/ 6" GRK-R4 SCREWS.
4. 2X6 CEILING JOISTS @ 24" O.C., ATTACH TO BEAM W/ (2) 4" GRK-R4 SCREWS.
5. TRIPLE (3) 2X6 RAFTER/CEILING JOIST BEARING BEAM (TYP.), ATTACH TO COLUMN W/ (2) SIMPSON LPC4Z OR APPROVED EQUAL.
6. MULTIPLE PLY 2X6 LAMINATED COLUMN WITH #4 BAR ANCHOR ROD, SEE DETAIL S301-03.
7. 4" THICK CONCRETE SLAB w/ 6x6 W2.9 x W2.9 WWF ON 6 MIL POLY VAPOR BARRIER ON 4" COMPACTED GRANNULAR FILL.
8. 2X6 BEARING LEDGER.
9. 2x6 TRUSS CROSS BRACING.
10. 2X6 HORIZONTAL BRIDGING.
11. PRE-ENGINEERED A TRUSSES @ 7'6" O.C. SEE DETAIL: S302-02
12. 2x4 BYPASS PURLINS AT 24" O.C. UP THE SLOPE.
13. METAL ROOFING
14. MECHANICAL PLATFORM, SEE DETAILS: S302-06 & S305-02.
15. (4) 2X6 COLUMN W/ (2) 2X6'S BEARING ON TRUSS AND (1) FULL HEIGHT 2X6 EACH SIDE OF TRUSS WEB MEMBER
16. 2X6 JOISTS @ 16" O.C.
17. (2) 2X6 BEAM
18. 6" THICK X 12" DIAMETER CONCRETE FOOTING, SEE DETAIL: S301-01 SIMILAR.

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SUBMITTAL DATE
12/23/20

A201265

COVERED WALK & MECHANICAL PLATFORM DETAILS

S305